REPAIR REPORT FOR:

COUNTY DITCH NO. 24 REPAIR: FAIRBAULT COUNTY, MINNESOTA

JUNE 14, 2021 Project No. 19-23499

REPORT FOR:

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Signature Sheet

I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

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County Ditch No. 24 Repair Faribault County, Minnesota

Engineer's Project Number: 19-23499

Dated this 14th day of June, 2021



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REPAIR PETITION

A repair petition was filed to address items currently in disrepair in Faribault County Ditch No. 24 (CD 24). Items listed within the repair petition include:

- Address continued erosion concerns through open ditch
- Repair sloughs
- Repair drop intake outlet
- Protect driveway entrance
- Straighten culvert under 338th Avenue

The Faribault County Drainage Authority approved the repair petition and appointed ISG as the engineer to evaluate the existing condition and propose repairs and repair alternatives to address maintenance items and erosion issues. The order added no further requirements.

LOCATION + WATERSHED

Faribault County Ditch No. 24 lies within Winnebago City Township of Faribault County in northwest Faribault County. The watershed provides drainage to approximately 970 acres and includes land from Sections 8, 9, 10, 16 and 17 of Winnebago City Township. Elevations within the watershed range from approximately 996 to 1062 Mean Sea Level (MSL). Faribault County Ditch No. 24 serves as the outlet for Faribault Judicial County Ditch No. 201 (JCD 201). JCD 201 drains approximately 2,270 acres; in turn, CD 24 serves as the outlet for a total of 3,240 acres.

The hydrological soil classification of CD 24 watershed is predominantly Type "C/D" soils which are considered prime for farmland, if adequately drained based on Natural Resources Conservation Service (NRCS) web soil survey. On-site soil analysis took place to confirm the soil textures present within the open ditch channel. Thirty-six soil samples were taken with a hand auger strategically that varied in depth and location within the open ditch channel. The soil samples confirmed soil texture present range from silty clay loam to silty clay; soils that are prone to erosion. Soil texture was a design consideration when determining solutions to erosion issues.

The system outlets through a series of berms, drop structures, and outlet pipes to control flow through an existing ravine to the Blue Earth River. Upstream of the ravine there is 1,800-feet of open ditch in Section 9 of Winnebago City Township. The open ditch contains a riprap spillway at the upstream portion before is crosses 338th Avenue with a 66-inch culvert. The repair items outlined in the petition are mainly within this portion of the system. In addition, CD 24 includes approximately 17,730 feet of subsurface drainage tile, and 9,000 feet of grass waterway. Complete maps of the existing conditions for the CD 24 watershed can be found in Appendix C.

HISTORY

Faribault County Ditch No. 24 was first constructed in 1915 as a subsurface tile system. In 1959, the system was improved with the addition of 1,800 feet of open ditch at the outlet of the system and 9,000 feet of grass waterway. In 2013, a repair was ordered which consisted of open ditch cleaning, partial Mainline tile replacement, crossing repairs, implementation of alternative side inlets, and installation of an outlet structure for ravine stabilization. The outlet structure consisted of a 60-inch outlet in the ravine with a 96-inch drop intake towards the inlet of the 60-inch pipe. Another 96-inch drop intake was installed further downstream to handle the flows during the higher rainfall events. This repair was completed to covey the flow through structures rather than overland down the ravine which has caused severe erosion and bluff instability over the years. Construction of the ravine repairs occurred in 2014 and 2015.

An improvement was petitioned in 2016 to improve the tile capacities throughout of the system. The Faribault County Drainage Authority approved the Final Engineers Report (FER), although the project is still in litigation. No resolution has been reached to this date.



Repairs to the downstream portions of the waterway and open ditch were petitioned in 2016 with a repair report developed later that year. The repairs included cleaning the open ditch, installing a 42-inch control structure in the outlet of the open ditch prior to flowing into the ravine structures, installing a rip rap spillway to connect the waterway to the open ditch, installing two field crossings, and repairing 1,170-feet of tile along Branch B. Construction of the repairs occurred in 2017.

REPAIR ITEMS

The information has been prepared from the original CD 24 profile drawings and alignment maps provided by Faribault County in conjunction with the additional records from historical repairs and improvements. A topographic survey of the open ditch portions of the open ditch was done by ISG in July of 2019 to address the repairs outlined in the repair petition. A close representation of the CD 24 watershed was created using the survey information in conjunction with LiDAR contours and viewers watershed boundaries provided from the most up-to-date redetermination of benefits. A map illustrating the existing CD 24 system can be found in Appendix C.

Outlined below is the existing condition of the items addressed in the repair petition followed by the proposed repairs or repair alternatives.

Sloughing

There are multiple areas in the open ditch portion of CD24 on the east side of 338th Avenue that are having bank failures. There are several sloughed areas which are causing material to fall into the open ditch and are causing or may have the potential for flow restrictions. There has been natural widening of the ditch caused by high, erosive flows and some downcutting in the past; therefore, the landowners directly adjacent to the ditch have lost productive cropland due to erosion and bank failures.

The sloughs range in size and severity, and the repairs to each slough will be addressed individually. The sloughs recommended to be repair have been numbered 1 through 5. A Slough Location map in Appendix C displays the location and extent of each slough. The slough repair areas were identified as slough that have potential for causing major flow restrictions, sediment export to downstream waters, bank instability, or potential for loss of farmland adjacent to open ditch. Areas that have naturally stabilized will not be addressed in the repair as it may cause more harm than benefit to disturb stabilized areas.

Slough #1, #2, #3, and #4 will be repaired with a bench side slope slough repair where the bench will be near the toe of the ditch. The benched side slope slough repair will create a more stable design for the open ditch. The bench is placed at the toe for these repairs because the sloughed material has naturally formed a bench at this location. The existing edge of buffer will remain in the same location therefore no land acquisition is expected. However, excavation from the repair will expand out into the adjacent fields which will require temporary damages. The buffer will be lowered down to take pressure off of the side slope and toe of ditch. The slide slopes will be flattened to 2:1 or 3:1 side slope ratio depending on the existing side slope ratios. Once the naturally formed bench elevation is reached, the side slope will merge into it. The bench widths will differ throughout each slough repair. Riprap will be placed at the bench to ensure there isn't any additional sloughing in the future and protect the toes from the high velocity flows.

Slough #5 is located on the south side of the ditch directly downstream of 338th Avenue. The landowner uses this area as a field access and due to sloughing is in jeopardy of damage. There is minor sloughing near the top of the ditch side slope. To be proactive in protecting the field access from erosive flows from the 338th Avenue, erosion control blanket will be installed near the top of the ditch and riprap will be installed at the toe. The riprap will help to stabilize and prevent any further sloughing into the open ditch.

Below are figures that display the location and existing conditions of the sloughs. Cross sections are available in Appendix A with the preliminary construction plans that display the proposed repairs for each individual slough.



It should be noted that the slough repairs noted are not permanent fixes for the open ditch portion of CD 24. The open ditch is still expected to need routine maintenance and erosion control repairs. A more compressive design for the open ditch will provide a more stable channel long term. A comprehensive design is more expensive for initial construction; however, it may reduce routine maintenance cost in the future.



Figure 1: Slough Locations

Table 1: Slough Overview

	Faribault County Ditch No. 24						
		Open Ditch Bank S	Stability				
Slough Overview	Location	Solution	Start Station	End Station	Total Length (LF)	Temporary Damages (AC)	Riprap (CY)
Slough #1	North	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.	5+30	6+75	145	0.11	43
Slough #2	North	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.	8+00	10+15	215	0.17	48
Slough #3	South	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.	11+00	11+95	95	0.07	21
Slough #4	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.		14+50	15+45	95	0.07	32
Slough #5	South	Re-sloping near top of ditch to prevent further sloughing & instabilities. Riprap will be placed along top of ditch to erosion control from 388th Avenue crossing	15+75	16+25	50	NA	50



Figure 2: Slough #1 viewing downstream



Figure 3: Slough #1 viewing upstream



Figure 4: Slough #2 viewing downstream



Figure 5: Slough #2 viewing upstream





Figure 7: Slough #3 viewing upstream



Figure 8: Slough #4 viewing downstream



Figure 9: Slough #4 viewing upstream



Figure 10: Slough #5



Figure 11: Slough #5 viewing upstream

Drop Intake Outlet

It is proposed to remove and replace the existing drop intake with a 24-inch alternative side inlet (ASI). The outlet of the ASI will be above the proposed bench for Slough #3 to ensure a free outlet. Benching of side slopes and spoil placement from Slough #3 will be taking place in the area of the ASI and will



direct more surface water to the inlet. Therefore, the ASI will be upsized from an 18-inch to a 24-inch intake to adequately convey surface flow.

338th Avenue Culvert

The existing 66-inch corrugated metal culvert under 338th Avenue connects the CD 24 open ditch diagonally across the road. Due to the angle in which the culvert crosses the road, the outlet on the east side of the culvert causes erosion along open ditch banks. The township has discussed independently to install a bend in the culvert to safely route water to the center of the open ditch and away from the ditch bank. However, this has not been completed to date. Extra rip rap has been added to the Slough #5 repair to add support to the bank and direct flow towards the center of the open ditch.



Figure 12: 338th Avenue Culvert

Seeding

All areas exposed during construction will be reseeded to provide bank stability. Erosion control blanket will be required on all slough repairs to provide fast vegetation establishment and protection from erosion. Seeding will be required to be completed within 2-days after final grading of the slough repair.

Tree Removals

There are approximately 11 trees within the open ditch or within the 16.5-foot buffer area. Trees in sloughed area in which earthwork and re-shaping of channel geometry will be taking place will be removed. In total, 6 trees are proposed to be removed as part of this repair. The remaining trees will be left in place as to not create additional instabilities in the open ditch channel.

ALTERNATIVE REPAIR OPTIONS

The above repairs listed specifically address the petitioned repair items and slough repair areas. Since the slough repairs comprise a large portion of the ditch and are also a band aid approach; other repair options should be considered to address the entire open ditch at one time and to factor in the ditch soil stability.

The limiting velocity method was used to determine suitable velocities with corresponding cross sections throughout the ditch. The Limiting Velocity Method determines a maximum recommended velocity based



on the type of soil present. For CD 24, soil textures along the open ditch were identified as silty clay loam from the Natural Resources Conservation Services (NRCS) Web Soil Survey (WSS). Using Table 2 the permissible velocity for silty clay loam soil texture is 3.5 feet per second when fair vegetation is present. This was the design factor for the given conditions on the CD 24 open ditch.

Table 2: NRCS Permissible Velocity

			Permissible velo	ecity	
	Bare		Channel	Vegetation	Condition
Soil Texture	channel	Retardance*	Poor	Fair	Good
	rn/s (ft/s)		(ftls)		
Sandy, silt,		В			
			0.61(2.0)	0.91 (3.0)	1.22 4.0)
sandy loam,	0.45 (1.5)	C	0.45 (1.5)	0.76(2.5)	1.07 (3.5)
and silty loam		D	0.45 (1.5)	0.61 (2.0)	0.91 (3.0)
Silty clay loam and	0.61 (2.0)	В	0.91 (3.0)	1.22 (4.0)	1.52 (5.0)
sandy clay loam	1000	C	0.76(2.5)	1.07 (3.5)	1.37 5.0)
D		D	0.61(2.0)	0.91 (3.0)	1.22 (4.0)
		В	1.07 (3.5)	1.52 (5.0)	1.83 (6.0)
Clay	0.76(2.5)	C	0.91 (3.0)	1.37 (4.5)	1.68 (5.5)
D			0.76(2.5)	1.22 (4.0)	1.52 (5.0)
Coarse Gravel	1.52 (5.0)	B, C, orD	1.52 (5.0)	1.83 (6.0)	2.13 (7.0)

Cobbles and shale 1.83 (6.0) B, C, orD 1.83 (6.0) 2.13 (7.0) 2.44(8.0)

Figure 7. Permissible velocities for diversions

Reference: USDA, NRCS Part 354 Stream Restoration Design, National Engineering Handbook, Chapter 8, Threshold Channel Design

Several different bank side slope configurations were modeled using XP SWMM, a 2-dimensional software used for hydrological and hydraulic modeling. Exports from the model show the anticipated channel velocities for each cross-section configuration. As shown in Table 3, the flatter the side slope; the lower the channel velocity. Also shown in the table are two-stage ditch configurations. This includes an inner (low flow) channel with benches serving as floodplains. The banks of the two-stage options were designed at 3:1. Figure 13 shows an example cross section of a two-stage ditch and side slope flattened to 5:1 along the CD 24 open ditch while Appendix C contains more cross sections of the two-stage and 5:1 channel options.

Table 3: Permissible Velocity Analysis

Velocity Comparisons (ft/s)							
	Evisting	Flatten Side	Flatten Side	Flatten Side	Flatten Side	Two-Stage	Two-Stage
	Existing	Slopes	Slopes	Slopes	Slopes	Ditch	Ditch
	2:1	3:1	4:1	5:1	6:1	15 - ft bench	20-ft bench
2yr	3.99	3.35	3.11	3.01	2.93	3.20	3.20
5yr	4.47	3.71	3.37	3.23	3.16	3.19	3.19
10yr	4.56	3.78	3.49	3.40	3.29	3.21	3.21
25yr	4.71	3.88	3.54	3.40	3.32	3.30	3.16
50yr	4.80	3.95	3.60	3.42	3.35	3.41	3.17
100yr	4.88	4.04	3.64	3.46	3.38	3.46	3.25

^{*}The choice of retardance B, C, or D will depend on the vegetation and maintenance planned for the diversion channel. Refer to the Handbook for Channel Design, SCS-TP-61, or similar information in the field office technical gUide, to select the vegetal retardance.



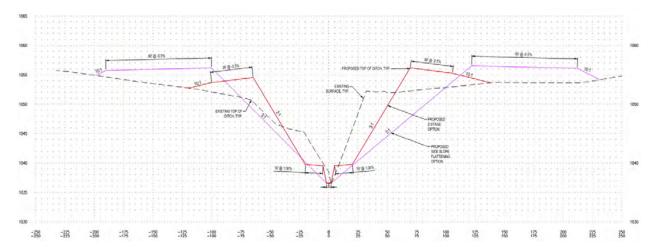


Figure 13: Two-Stage and 5:1 Cross Section

Option 1:Flatten Side Slopes to 5:1

Based on the results of the XP SWMM model and the anticipated channel velocities; Option 1 includes flattening ditch side slopes to a 5:1 slope for bank stability. Currently the CD 24 banks are at a side slope ratio of 2:1 and are not stable for the velocities flowing through the open ditch. Flattening the side slopes to a 5:1 ratio provides a channel velocity of under 3.5 feet per second for all modeled rainfall events.

In order to construct the ditch to a 5:1 side slope, land acquisition is needed to flatten the top of the ditch bank and extend the 1-rod buffer laterally away from the top of the ditch. Additionally, spoils from excavation will be spread out away from the ditch bank which will require temporary damages to those lands for disturbance. In Option 1, there are approximately 3.75 acres of land acquisition needed and 5.29 acres of temporary impacts for spoil placement. On average, the new top of ditch with is 225-feet wide not include the buffer on each side. The ditch side slopes of 5:1 will allow for an excavator to crawl down along the side for future ditch maintenance.

Option 2:Two-Stage Ditch

Option 2 considers converting the open ditch east of 338th Avenue into a two-stage ditch. The two-stage is designed with an inner channel width of 4-feet with 1:1 side slope ratio for a depth of 3-feet. The inner channel was designed to hold the 1.5-year storm event and also to utilize the existing channel as much as possible. The benches were designed to be 2 to 3 times the width of the top of the inner channel (7-feet). Each bench is proposed at 15-feet where it transitions back to a 3:1 side slope up to the top of the ditch. The new top of ditch width is on average 140-feet wide. The two-stage ditch alternatives were designed to follow design standards set by NRCS in the National Engineering Handbook – Part 654 for Two-Stage Ditch Design in combination with recommendations from the Department of Natural Resources Geomorphology team.

Since the two-stage ditch utilizes portions of the existing open ditch and has a 3:1 side slope; the overall footprint of the channel is much less that Option 1. Permanent damages are estimated at 1.49-acres while temporary damages are estimated at 2.76-acres.

Given the proposed geometry of the two-stage ditch and the deep ditch; a long reach excavator will not be able to maintain the open channel from the new top of ditch bank. A ramp off of 338th Avenue will be constructed to access the bench of the two-stage ditch for maintenance to the open channel. This will allow for smaller equipment (mini-excavator, tractor backhoe, etc.) to clean the inner channel of the open ditch. Cleaning frequencies are much less with the natural two-stage ditch cross section; therefore, maintenance is expected to be periodic spot cleaning.



Option 3:42-Inch Pipe with Waterway

Option 3 proposes to replace the open ditch east of 338th Avenue with a 42-inch pipe and waterway over the top. The existing 66-inch culvert would remain in place and flow directly into the 42-inch pipe. The ditch would be filled in to provide 4-feet of cover over the top of the pipe for protection.

A rip rap overflow would be built at the headwall of the pipe apron and waterway beginning. This will provide protection of the pipe and waterway transition and also allow peak water levels to flow over and utilize the waterway for overflow. The headwall and 42-inch pipe will be placed downstream of Slough #5 so that slough can be repaired and to provide space for flow from the culvert to enter into the pipe without backwater. The headwall will be placed 90 feet downstream of the 338th Avenue culvert. At this location, the invert elevation (matching legal ditch grade) is 1036.87.

In lieu of a round pipe, a 42-inch arch equivalent pipe (31-inch \times 51-inch span) would provide a lower the overall bottom elevation of the waterway overflow to lessen upstream impacts on the west side of 338th Avenue. The difference in height between a 42-inch RCP and 51-inch span arch pipe is 1.54 feet which has significant impacts upstream on the CD 24 waterway.

At the headwall beginning, the top of the 51-inch span pipe would be at elevation 1039.83 which brings the 4-foot cover elevation to 1043.83. If a 42-inch round RCP was used, the elevation after accounting for 4-feet of cover would be at 1045.37. The legal rip rap spillway elevation at the end of the waterway is at elevation 1044.95. Using a 51-inch span arch pipe provides 1.12-feet of freeboard while using a 42-inch round RCP would be 0.42-feet above the rip rap spillway elevation.

The proposed pipe would be placed a 0.2% matching the legal ditch grade and elevation. The pipe would be required to have a minimum of 6-inches of crushed rock as foundation. At the end of the pipe-waterway combination; a rip rap spillway will be constructed to transition the waterway flow back into the 42-inch pipe control structure. The open ditch control structure will remain in place with an emergency trash grate at elevation 1045.00.

The installation of a pipe would eliminate the potential for open ditch sloughing, erosion, and sediment delivery downstream. The pipe would also control flow at the outlet of CD 24. The filling of the open ditch would not require land acquisition, and the open ditch would be filled in with an overflow waterway.

Although, there would be considerable benefits at the outlet of the system in terms of bank stabilization and water quality, the reduction in flow and water storage volume in the system increases flood extents and water elevations to upstream landowners. Table 4 below compares the maximum flood extents for each option under all modeled rainfall frequencies for each landowner impacted. As shown, there are increases in flooded areas under the 5-year through 25-year events ranging from 0.4-acres to 5.2-acres. Under these frequencies, the filled open ditch and pipe do back up water given the restriction and timing of flow to the open ditch.

Under the 2-year event, the improvement tile carries flow to the open ditch faster than the existing conditions which reduces the overall flow in the waterway and open ditch. Under the extreme events (50-year and 100-year), flow is no longer restricted by the pipe and waterway and the culvert controls the flows to the open ditch. With the timing change from the improvement tile, there is less water that uses the waterway for overland flow and ultimately there is a reduction in flooding.



Table 4: Area Flooded Comparison

Lacation	2-	Year (acre	s)	5-Year (acres)			10-Year (acres)		
Location	Existing	Pipe	Arch	Existing	Pipe	Arch	Existing	Pipe	Arch
Carlson	10.2	10.2	10.2	15.4	15.9	15.6	16.8	18.3	18.1
Cole	2.4	2.4	2.4	2.5	2.5	2.5	2.7	2.7	2.7
Rynearson	4.4	4.4	4.4	9.6	9.8	9.7	12.0	12.2	11.0
Total	17.0	17.0	17.0	27.4	28.2	27.8	31.4	33.2	31.7
Landina	25-Year (acres)			50-Year (acres)			100-Year (acres)		
Location	Existing	Pipe	Arch	Existing	Pipe	Arch	Existing	Pipe	Arch
Carlson	21.7	25.2	23.5	31.2	31.1	31.0	35.0	34.6	34.5
Cole	2.8	3.9	3.2	5.6	5.6	5.6	7.6	7.5	7.5
Rynearson	14.3	14.9	14.3	19.2	18.3	17.2	27.8	26.5	25.8
Total	38.8	44.0	41.0	56.0	55.0	53.7	70.3	68.6	67.8

There are 3 alternatives to Option 3 which include a 31-inch x 51-inch arch pipe; a 42-inch round RCP; and a 42-inch round dual wall HDPE pipe. Given the potential large cost estimate differences, if Option 3 is selected it is recommended to bid all three option as alternates to determine the actual bid price of each alternative.

COST ESTIMATES

Cost estimates have been generated for the petitioned repair items and all other options for repair. For Options 1 through Option 3B, the repairs to the west side of 338th Avenue, and tree removals have been included in the cost estimates to cover the repair petition. Table 5 summarizes the cost estimates for each option while detailed cost estimates are included in Appendix D.

Table 5. Cost Estimate Summary

Repair Option	Estimated Cost
PETITIONED REPAIR	\$ 126,105
OPTION 1: 5:1 SIDE SLOPES	\$ 324,591
OPTION 2: TWO-STAGE DITCH	\$ 206,906
OPTION 3: 31" X 51" ARCH PIPE	\$ 431,871
OPTION 3A: 42" RCP PIPE	\$ 345,824
OPTION 3B: 42" DUAL WALL PIPE	\$ 244,086



It should be noted that the unit prices for the cost estimate are based on previously constructed project with similar scope of work. Based on previous projects, the proposed repairs to the CD 24 system described in this report are cost effective.

MULTI-PURPOSE DRAINAGE MANAGEMENT

Multi-purpose drainage management incorporates Best Management Practices (BMPs) which utilize effective measures aimed at reducing sediment and nutrient loading and improving water quality. These BMPs are divided into the following three areas.

Preventative Measures

Preventative measures that can be applied throughout the watershed include crop rotation, cover crops, residue management, and nutrient management. These measures are aimed at controlling sediment, minimizing erosion and nutrient loss, and sustaining the soils health, all without dramatically changing the current land use of the landscape.

Control Measures

Control measures are practices aimed at improving water quality directly associated with the flow of water by reducing peak flow and providing in-stream storage, sedimentation, and nutrient uptake. Examples of control measures include alternative tile intakes, grassed waterways, two stage ditches, water control structures, and controlled subsurface drainage. These practices are directly linked to the conveyance of subsurface tile water or open channel ditch flow.

Treatment Measures

The function of treatment measures is to improve water quality by directly removing sediment and nutrients from the subsurface or surface water flow throughout a watershed. Examples of treatment measures include surge basins (storage ponds), filter/buffer strips, wetland restorations, woodchip bioreactors, and water and sediment control basins (WASCOBs). These practices may be incorporated to either the public or private drainage systems.

Conservative Drainage Practices

Conservative drainage practices, such as construction of controlled drainage systems, provide an option for improving the water quality within a drainage system. Through utilization of control structures, these systems are designed to allow agricultural producers to regulate water levels in their fields. The water level in the ground can be lowered during planting and harvest seasons and allowed to rise during the growing season. Water and nutrients stored in the soil during the growing season can then be used by the crops during drier periods, potentially increasing yields.

Funding

There are several outside sources of funding to potentially help pay for water quality improvements implemented in a drainage system such as this. A main source of funding for this type of project is through the Minnesota Board of Water and Soil Resources (BWSR) Clean Water Fund (CWF). The primary purpose of activities funded with grants associated with the CWF is to restore, protect and enhance water quality. One CWF grant program is the Multipurpose Drainage Management Grant. This grant is geared towards implementing practices that will reduce the transport of sediment and nutrient loads. Some practices that have been funded in the past include grade stabilization, grassed waterways, water and sediment control basins, alternative side inlets, saturated buffers, storage wetlands, denitrifying bioreactors, etc.

Another potential source is the Legislative-Citizen Commission on Minnesota Resources (LCCMR) Environment and Natural Resources Trust Fund (ENRTF) which was established to provide funding for



activities that protect, conserve, preserve, and enhance Minnesota's "air, water, land, fish, wildlife, and other natural resources." The LCCMR prioritizes innovative ideas that provide multiple benefits.

If landowners are interested in pursuing practices that go beyond this project scope, a few programs may be a source for funding. The Agriculture Best Management Practices (BMP) Loan Program provides loans to rural landowners to encourage BMPs that help counteract pollution problems.

Another option for individual landowners that are interested in pursuing additional practices is the Environmental Quality Incentives Program (EQIP) is a voluntary program through the NRCS that provides financial assistance to individual landowners for various conservative practices as identified above.

In addition, the BWSR Community Partners Grant may be an option. This grant leverages the interest of non-governmental partners such as lake and river associations, boy/girl scout troops and other civic groups to install on-the ground projects that reduce runoff and keep water on the land. It also allows for multiple local government units to work together on a project that involves the Community Partners Grant. Projects installed with the Community Partners Grant are intended to be structural or vegetative practices designed to reduce runoff and/or keep water on the land.

All of the water quality measures proposed with this project are applicable for some source of outside funding. The sources listed above are grants that could be a good fit for this project and if the timing of the project works in conjunction with the grant schedule. These grants can be applied for, if there is support from the drainage authority and/or interest from landowners. Additional water quality measures can be discussed in further detail, if requested.

CONCLUSIONS + RECOMMENDATIONS

The repair items outlined in the repair petition have been deemed to be in disrepair. There are many areas in which sloughing has widened the top of the open ditch and encroached on the adjacent landowner's property. Sloughing has also caused sediment to fall into the flow line of the ditch causing flow restrictions and water quality concerns. It is the opinion of the engineer that a hearing be held to review the options with the Drainage Authority and landowners to determine a repair option.

APPENDIX A: PRELIMINARY CONSTRUCTION PLANS

FARIBAULT COUNTY DITCH 24 REPAIR

FARIBAULT COUNTY, MINNESOTA

REPAIR PLANS

ISG PROJECT # 19-23499

LEGEND

EXISTING

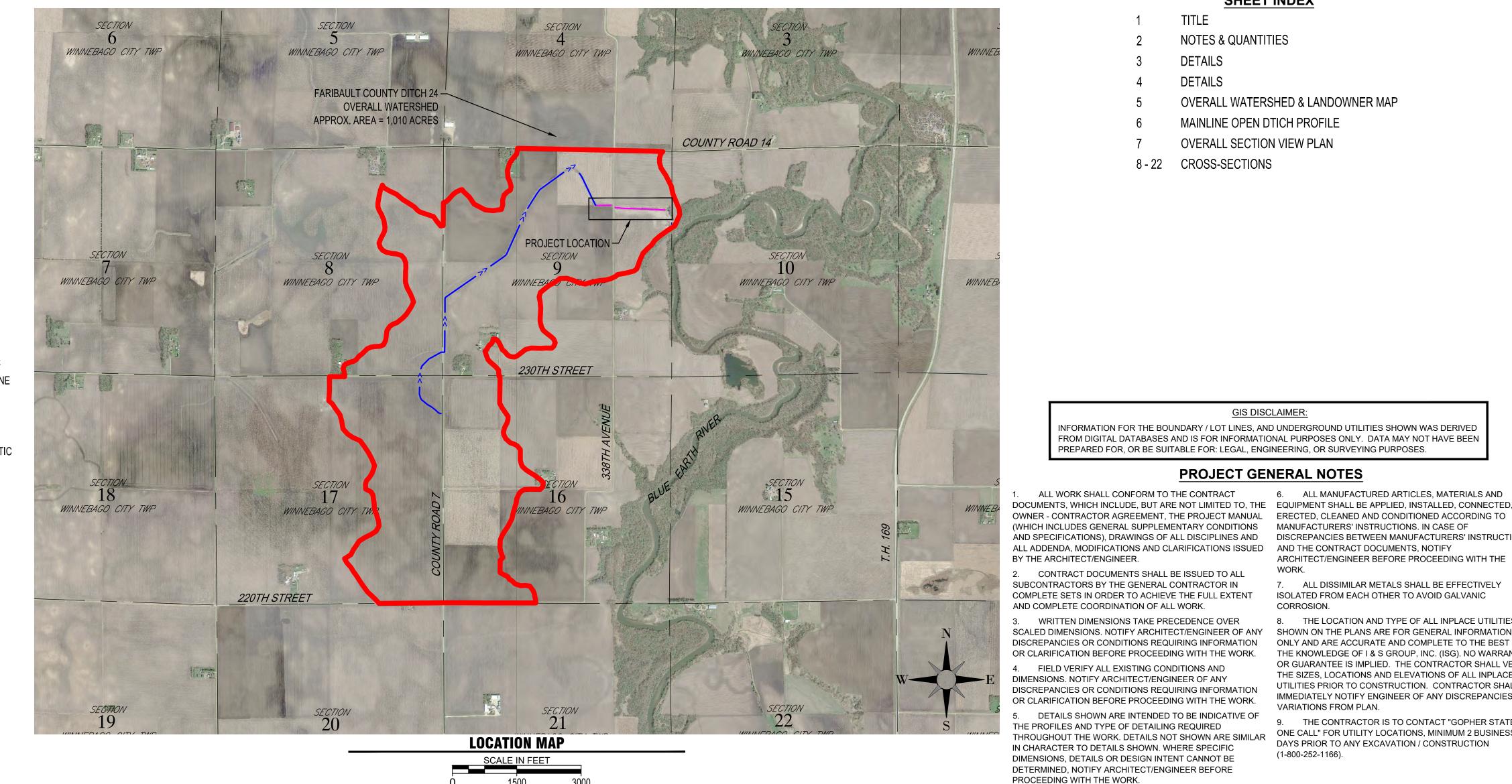
UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE

DECIDUOUS TREE **CONIFEROUS TREE** TREE LINE DROP INTAKE **HYDRANT**

POWER POLE

PROPOSED

PROPOSED OPEN DITCH OPEN DITCH REPAIR CULVERT (RCP) CULVERT (CMP) OVERHEAD ELECTRIC UNDERGROUND ELECTRIC UNDERGROUND TV CONTOUR (MINOR) DROP INTAKE SLOUGH REPAIR SPOIL PLACEMENT TREE CLEARING REMOVE TREE



SHEET INDEX

- **NOTES & QUANTITIES**
- **DETAILS**
- DETAILS
- OVERALL WATERSHED & LANDOWNER MAP

INFORMATION FOR THE BOUNDARY / LOT LINES, AND UNDERGROUND UTILITIES SHOWN WAS DERIVED FROM DIGITAL DATABASES AND IS FOR INFORMATIONAL PURPOSES ONLY. DATA MAY NOT HAVE BEEN

PROJECT GENERAL NOTES

PREPARED FOR, OR BE SUITABLE FOR: LEGAL, ENGINEERING, OR SURVEYING PURPOSES

- MAINLINE OPEN DTICH PROFILE
- OVERALL SECTION VIEW PLAN
- **CROSS-SECTIONS**

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PROJECT

FARIBAULT COUNTY

DITCH 24 REPAIR

ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE **FARIBAULT COUNTY** 7. ALL DISSIMILAR METALS SHALL BE EFFECTIVELY

8. THE LOCATION AND TYPE OF ALL INPLACE UTILITIES SHOWN ON THE PLANS ARE FOR GENERAL INFORMATION ONLY AND ARE ACCURATE AND COMPLETE TO THE BEST OF OR GUARANTEE IS IMPLIED. THE CONTRACTOR SHALL VERIF THE SIZES, LOCATIONS AND ELEVATIONS OF ALL INPLACE

ALL MANUFACTURED ARTICLES, MATERIALS AND

DISCREPANCIES BETWEEN MANUFACTURERS' INSTRUCTIONS

ISOLATED FROM EACH OTHER TO AVOID GALVANIC

THE CONTRACTOR IS TO CONTACT "GOPHER STATE ONE CALL" FOR UTILITY LOCATIONS, MINIMUM 2 BUSINESS DAYS PRIOR TO ANY EXCAVATION / CONSTRUCTION

MINNESOTA

VARIATIONS FROM PLAN.

(1-800-252-1166).

	REVISION SCHEDULE						
DATE]	DESCRIPTION	BY				
PROJECT NO.		19-23499					
FILE NAME		23499 TITLE					
DRAWN BY		SMW					
DESIGNE	D BY	SMW/ MAO					

PROJECT INDEX:

OWNER:

FARIBAULT COUNTY DRAINAGE AUTHORITY 415 S. GROVE STREET, SUITE 8 **BLUE EARTH, MN 56013** PH: (507) 526-2388

PROJECT ADDRESS / LOCATION:

SECTION: 4, 8-10, 16-17 WINNEBAGO CITY, TWP

FARIBAULT COUNTY, MINNESOTA

MANAGING OFFICE:

ISG

MANKATO OFFICE 115 EAST HICKORY STREET SUITE 300 **MANKATO, MN 56001** PHONE: 507.387.6651

FAX: 507.387.3583 PROJECT MANAGER: MARK ORIGER **EMAIL: MARK.ORIGER@ISGINC.COM**

SPECIFICATIONS REFERENCE

ALL CONSTRUCTION SHALL COMPLY WITH THE FARIBAULT COUNTY REQUIREMENTS AND MnDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION, 2018 EDITION, AND THE STANDARD SPECIFICATIONS FOR SANITARY SEWER, STORM DRAIN AND WATERMAIN AS PROPOSED BY THE CITY ENGINEERS ASSOCIATION OF MINNESOTA 2013, UNLESS DIRECTED OTHERWISE.

PROJECT DATUM

HORIZONTAL COORDINATES HAVE BEEN REFERENCED TO THE NORTH AMERICAN DATUM OF 1983 (NAD83), 1996 ADJUSTMENT (NAD83(1996)) ON THE FARIBAULT COUNTY COORDINATE SYSTEM, IN U.S. SURVEY FEET

ELEVATIONS HAVE BEEN REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88). RTK GPS METHODS WERE USED TO ESTABLISH HORIZONTAL AND VERTICAL COORDINATES FOR THIS PROJECT

B.M. ELEVATION = 1076.21

MnDOT MONUMENT - WINNEBAGO 6 MN043

TOPOGRAPHIC SURVEY

THIS PROJECT'S TOPOGRAPHIC SURVEY CONSISTS OF DATA COLLECTED IN JULY 2019 BY ISG.

TITLE

REVIEWED BY

TITLE

ORIGINAL ISSUE DATE --/--/--

CLIENT PROJECT NO.

OF 22

GENERAL OPEN DITCH NOTES:

- 1. DURING CONSTRUCTION, CONTRACTOR SHALL MAINTAIN A DRAINAGE OUTLET FOR THE ENTIRE FARIBAULT COUNTY DITCH 24 PROJECT AREA.
- 2. ALL PIPE DIMENSIONS REFERENCED IN THE PLANS REFER TO THE INSIDE DIAMETER.
- 3. UNLESS OTHERWISE NOTED, CONTRACTOR SHALL LIMIT CONSTRUCTION ACTIVITY TO WITHIN A 33-FOOT WIDE AREA ALONG TOP OF DITCH ALIGNMENTS. DISTURBANCE THROUGH ROAD CROSSINGS, ROAD DITCHES, AND GRASS BUFFERS SHALL BE LIMITED TO THE TRENCH WIDTH NECESSARY FOR SAFE CONSTRUCTION PRACTICES.
- 4. A 16.5-FOOT GRASS STRIP SHALL BE ESTABLISHED IN AREAS THAT DO NOT HAVE AN EXISTING 16.5-FOOT GRASS STRIP. SEEDING SHALL OCCUR AFTER ALL WORK HAS BEEN COMPLETED IN THE AREA AND SHALL COMPLY WITH THE CONTRACT DOCUMENTS. THESE AREAS WILL BE DETERMINED BY THE ENGINEER.
- 5. DITCH CLEANING SHALL BE PERFORMED ON THE SIDE OF THE DITCH THAT IS THE LOWEST FOR THE GREATEST DISTANCE ALONG THE OPEN DITCH SEGMENT. DITCH CLEANING SPOILS SHALL BE PLACED WITHIN 16.5-FOOT WIDE GRASS STRIP FROM THE TOP OF DITCH SLOPE UNLESS OTHERWISE DETERMINED BY THE ENGINEER.
- 6. TOPSOIL IN 50' WIDE BY 1' FOOT DEEP SPOIL AREAS AS SPECIFIED ON PLANS SHALL BE STRIPPED PRIOR TO SPOIL PLACEMENT.
- 7. SHAPING AROUND SIDE INLETS, WASCOBS, AND CULVERT INLETS SHALL BE INCIDENTAL TO THEIR RESPECTIVE PAY ITEMS.
- 8. ALL SPOIL LEVELING, GRADING, AND RESTORATION OF DISTURBED AREAS SHALL BE IN ACCORDANCE TO THE CONTRACT DOCUMENTS AND SHALL BE INCIDENTAL TO THE WORK PERFORMED.
- 9. ALL EXISTING TILE OUTLETS INTO THE OPEN DITCH, INCLUDING ANY NOT SHOWN ON THE PLANS, SHALL BE REPAIRED. UNLESS SPECIFICALLY NOTED, HDPE OR PVC SHALL BE ACCEPTABLE MATERIAL FOR ALL TILE REPAIRS (SEE DETAILS).
- 10. EXISTING TILE OUTLETS MAY BE SALVAGED, REUSED, AND PROTECTED WITH RIPRAP IF THE OUTLET IS DETERMINED TO BE IN GOOD CONDITION BY THE ENGINEER. TILE REPAIR AT THESE LOCATIONS SHALL BE PAID FOR AS PAY ITEM "ARMOR TILE OUTLET" (SEE DETAILS).
- 11. CONTRACTOR MUST NOTIFY ENGINEER OF ANY CULVERT SECTIONS DEEMED NOT SALVAGEABLE PRIOR TO REMOVAL AND SHALL BE ADDRESSED BEFORE CULVERT WORK IS DONE.
- 12. MISCELLANEOUS TREE CLEARING SHALL BE INCIDENTAL TO DITCH CLEANING PAY ITEM(S).
- 13. ALL TREE REMOVALS MUST BE COMPLETED BY MAY 1, 2020.

TOTAL ESTIMATED QUANTITIES						
Item Code	ltem	Unit	Estimated Quantity			
2021.501	MOBILIZATION	LS	1			
2101.511	CLEAR AND GRUB TREE (OVER 6")	EA	6			
2104.509	REMOVE EXISTING SIDE INTAKE	EA	1			
2105.602	AGGREGATE BASE (CV) (P), CLASS V (ACCESS ROAD)	CY	20			
2106.501	COMMON EXCAVATION	CY	2213			
2106.501	OPEN DITCH SIDE SLOPE DRESSING W/ TOP SOIL	CY	263			
2106.501	TOP SOIL STRIP & PLACE SPOILS	SY	3108			
2506.502	INSTALL 24-INCH ASI RISER ASSEMBLY W/TRASH GRATE	EA	1			
2506.502	INSTALL 24-INCH ASI OUTLET ASSEMBLY	EA	1			
2511.501	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	381			
	16.5' BUFFER STRIP SEEDING					
2575.501	(SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	SY	1531			
	STANDARD SIDESLOPE & BENCH SEEDING					
	(SEED MIX: BUFFER BLEND WITH Mn DOT CATEGORY 4					
2575.501	EROSION CONTROL BLANKET)	SY	3797			
2575.523	NON-WOVEN GEOTEXTILE FABRIC	SY	25			

ALTERNATIVE SIDE INLET (ASI)							
ASI #1	STATION	INTAKE TYPE	RISER SIZE (in)	RISER DEPTH (LF)	OUTLET SIZE (in)	OUTLET LENGTH (LF)	OUTLET GRADE (%)
1	12+00	TRASH GRATE	24	7	24	33	3.00

	Faribault County Ditch No. 24						
		Open Ditch Bank S	Stability				
Slough Overview	Location	Solution	Start Station	End Station	Total Length (LF)	Temporary Damages (AC)	Riprap (CY)
Slough #1	North	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.	5+30	6+75	145	0.11	43
Slough #2	North	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.	8+00	10+15	215	0.17	48
Slough #3	South	Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.	11+00	11+95	95	0.07	21
Slough #4	ough #4 South Benched side slope slough repair. Bench near toe of open ditch side slope where natural bench has formed from slumped material.		14+50	15+45	95	0.07	32
Slough #5	South	Re-sloping near top of ditch to prevent further sloughing & instabilities. Riprap will be placed along top of ditch to erosion control from 388th Avenue crossing	15+75	16+25	50	NA	50



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PROJECT

FARIBAULT COUNTY

DITCH 24 REPAIR

REVISION SCHEDULE

DATE DESCRIPTION BY

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FILE NAME	23499 DETAILS
DRAWN BY	SMW
DESIGNED BY	SMW/ MAO
REVIEWED BY	MAO
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CLIENT PROJECT NO	_

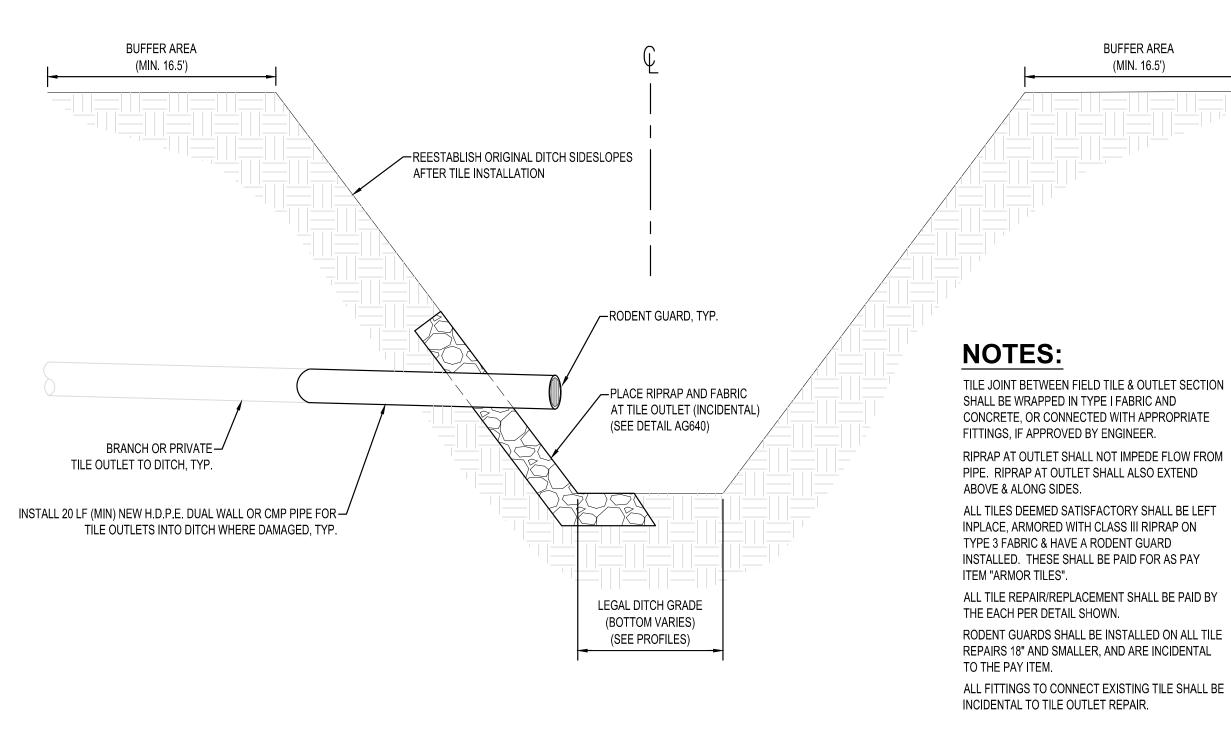
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NOTES & QUANTITIES

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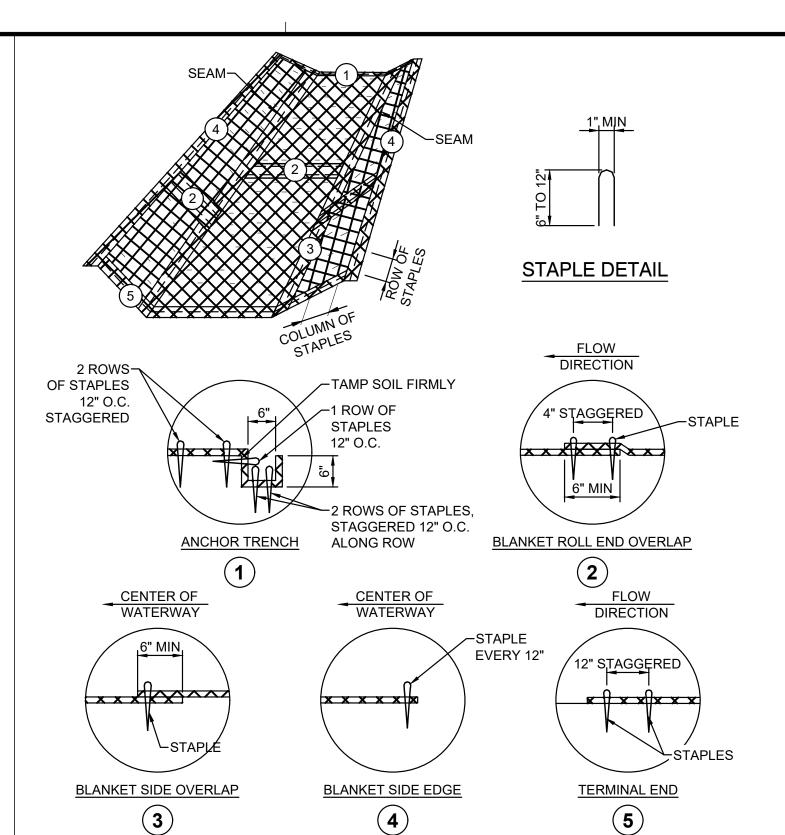
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NOTE: ALL TILE OUTLETS AND FIELD INTAKES SHALL BE REPAIRED OR PROTECTED



TYPICAL TILE OUTLET REPAIR

AG450



NOTES:

INSTALL EROSION CONTROL BLANKET (ECB) OVER WATERWAYS AS SHOWN IN THE STORM WATER POLLUTION PREVENTION PLAN.

THE ECB SHALL CONFORM TO MNDOT STANDARD SPECIFICATIONS SECTION 3885.

PREPARE SOIL PRIOR TO INSTALLING ECB, INCLUDING SEEDING AND FERTILIZING.

THE ECB SHALL BE PLACED IN FIRM CONTACT WITH THE SOIL AND NOT BE ALLOWED TO BRIDGE OVER SURFACE IRREGULARITIES. THE MAT SHALL NOT BE STRETCHED.

START LAYING THE MATS BY ROLLING CENTER MAT IN THE DIRECTION OF FLOW, CENTERED ON THE CENTERLINE OF WATERWAY. THERE SHALL NOT BE AN OVERLAP OF MATS AT THE CENTER OF THE WATERWAY.

THE ECB SHALL BE ANCHORED, OVERLAPPED, AND STAPLED ACCORDING TO MANUFACTURER'S INSTRUCTIONS. IF NO MANUFACTURER'S INSTRUCTIONS ARE AVAILABLE, INSTALL THE MAT AS FOLLOWS.

STAPLES SHALL BE "U" SHAPED, 0.12" DIAMETER WIRE OR GREATER (#11 GAUGE). (SEE STAPLE DETAIL FOR DIMENSIONS)

- BURY UPSTREAM END OF MAT IN A TRENCH 6" WIDE BY 6" DEEP AND STAPLED IN STAGGERED ROWS ACROSS THE WIDTH AS SHOWN IN DETAIL 1.
- C. FOR JOINING ENDS OF ROLLS, OVERLAP END OF UP SLOPE MAT A MINIMUM OF 6"

 OVER DOWN SLOPE MAT (SHINGLE STYLE). USE A DOUBLE ROW OF STAGGERED STAPLES 4" APART, AS SHOWN IN DETAIL 2.
- D. MATS ON SIDE SLOPES SHALL OVERLAP A MINIMUM OF 6" OVER THE MAT BELOW (SHINGLE STYLE). STAPLE OVERLAP AT 12" INTERVALS. (SEE DETAIL 3)
- E. THE OUTER EDGE ALONG SIDES OF THE MAT SHALL BE STAPLED EVERY 12". (SEE DETAIL 4)

F. STAPLES ARE TO BE PLACED ALTERNATELY IN COLUMNS (IN THE DIRECTION OF THE WATERWAY) 2' APART AND IN ROWS (ACROSS THE WATERWAY) 3' APART THROUGHOUT THE AREA COVERED BY THE ECB.

G. DOWNSTREAM (TERMINAL) END OF BLANKET SHALL BE STAPLED WITH A DOUBLE ROW OF STAGGERED STAPLES 12" APART. (SEE DETAIL 5)

DIMENSIONS

RIPRAP

4 CY CLASS III

8 CY CLASS III

12 CY CLASS III

PIPE DIA

12" TO 18"

30" TO 36"

ISG

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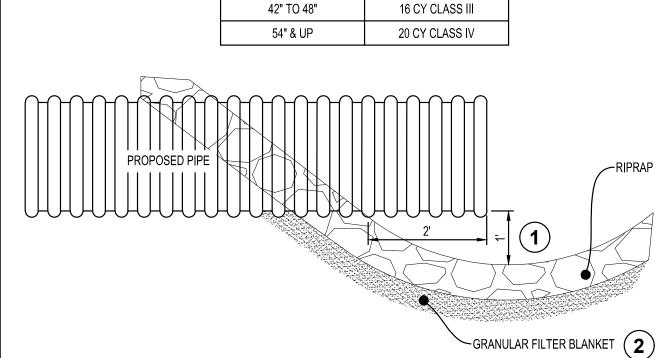
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DETAILS

CHEE

OF 22

CONSTRUCT 1' TALL BERM-GRADE BACKSIDE OF BERM TO DRAIN TO EXISTING SURFACE INTAKES SLOPED GRASS BUFFER (MIN. 16.5') EXCAVATED MATERIAL TO BE — PLACED IN TOPSOIL STRIPPING AND SOIL PLACEMENT AREA FLATTEN SIDE SLOPES — (SLOPES VARY) NOTE: CREATE BENCH-**EXCAVATION PAID PER CY** LENGTH WILL VARY PLACE CLASS III RIPRAP WITH-FABRIC AT BASE OF DITCH (KEY INTO SIDE SLOPE) LEGAL DITCH GRADE (BOTTOM VARIES) (SEE PROFILES) BENCHED SLOPE SLOUGH REPAIR WITH SLOPED BUFFER



NOTES:

EROSION CONTROL BLANKET INSTALLATION

1' MIN. ABOVE RIPRAP. FOR PIPES GREATER THAN OR EQUAL TO 30", USE 1.5' - 2'.

THE CONTRACTOR, AS AN OPTION, MAY SUBSTITUTE A GEOTEXTILE FABRIC, SPEC. 3601, FOR THE GRANULAR FILTER BLANKET. THE FABRIC SHOULD EXTEND BEYOND THE RIPRAP BY 3' IN ALL DIRECTIONS.

FOR PIPES LESS THAN 18", INSTALL RODENT GUARD (INCIDENTAL TO TILE OUTLET)

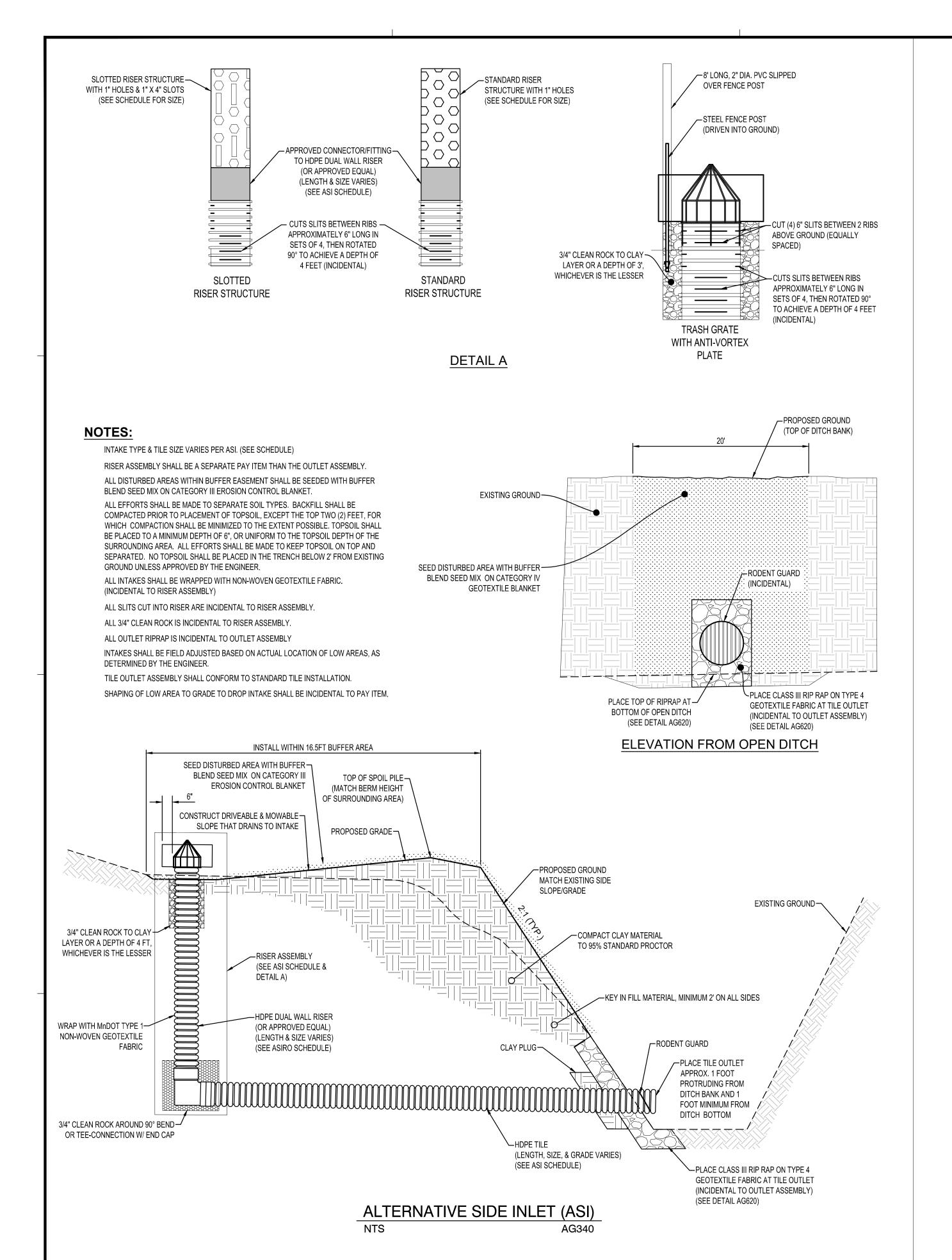
RIPRAP AT OUTLET SHALL NOT IMPEDE FLOW FROM PIPE, OR RECEIVING BODY. RIPRAP
AT OUTLET SHALL ALSO EXTEND ABOVE AND ALONG SIDES OF PIPE.

ADDITIONAL RIPRAP MAY BE NECESSARY BASED ON ELEVATIONS (PAID FOR BY CY)

RIPRAP AT TILE OUTLET

NTS AG620

SHEET





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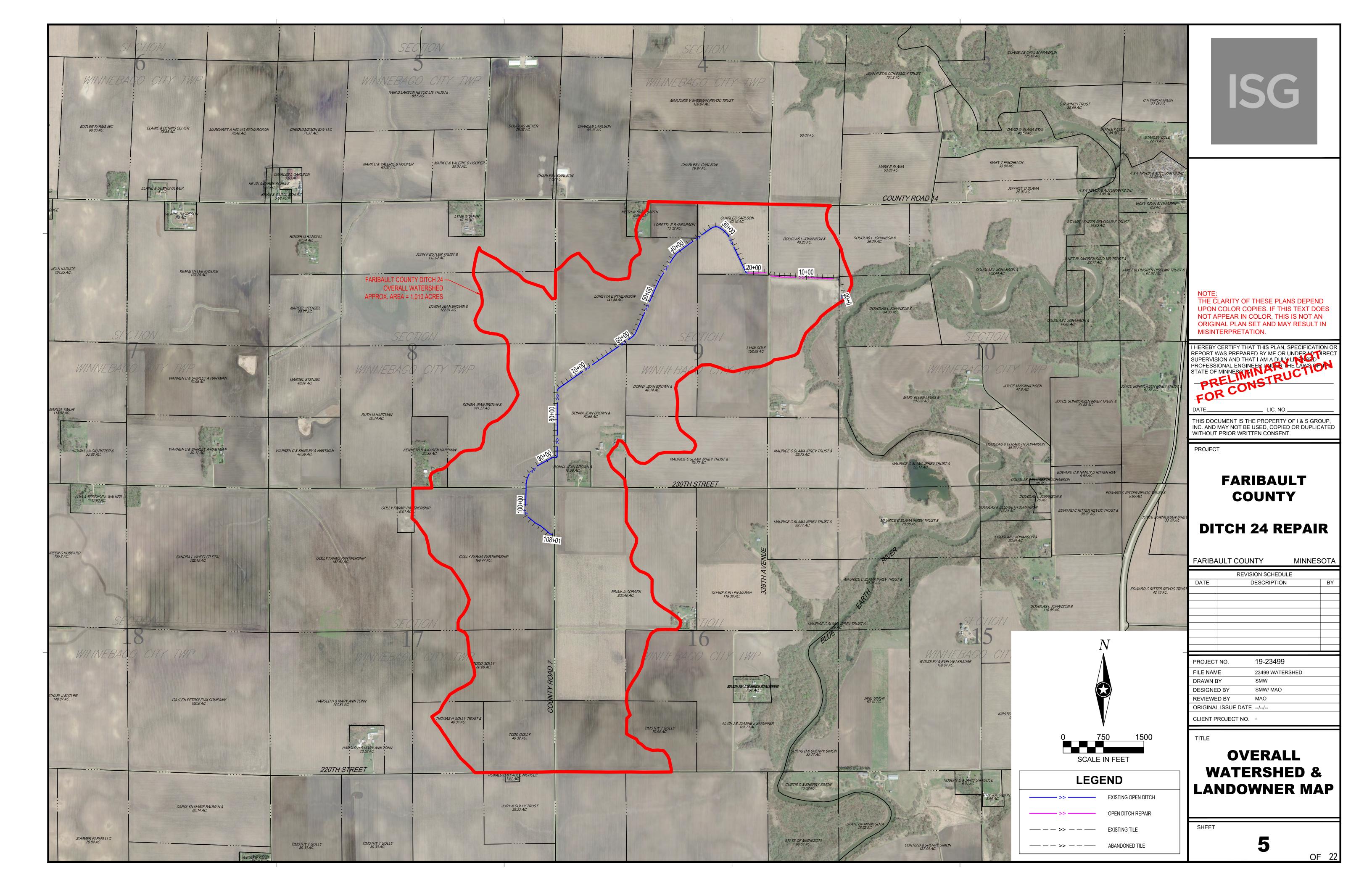
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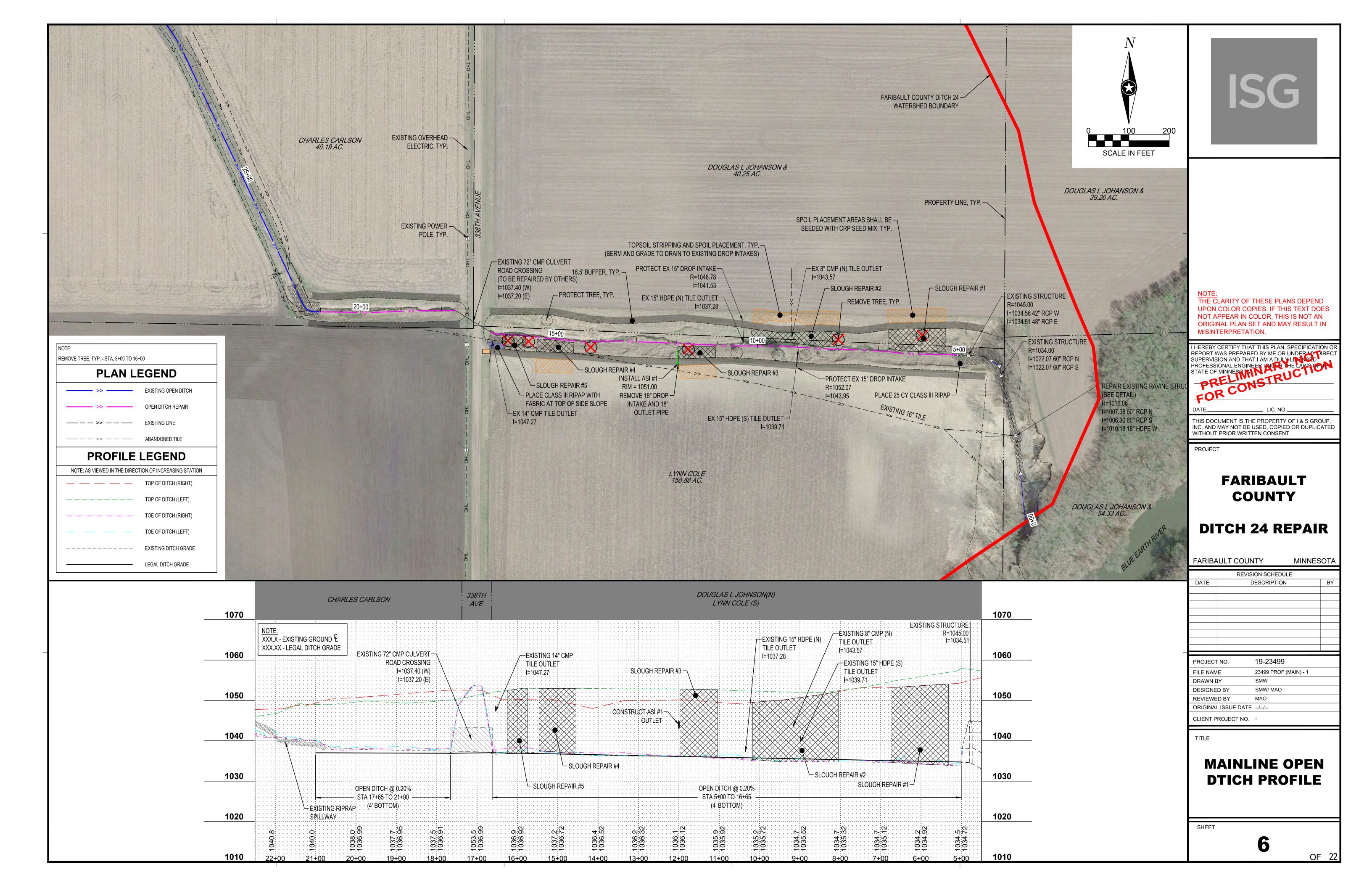
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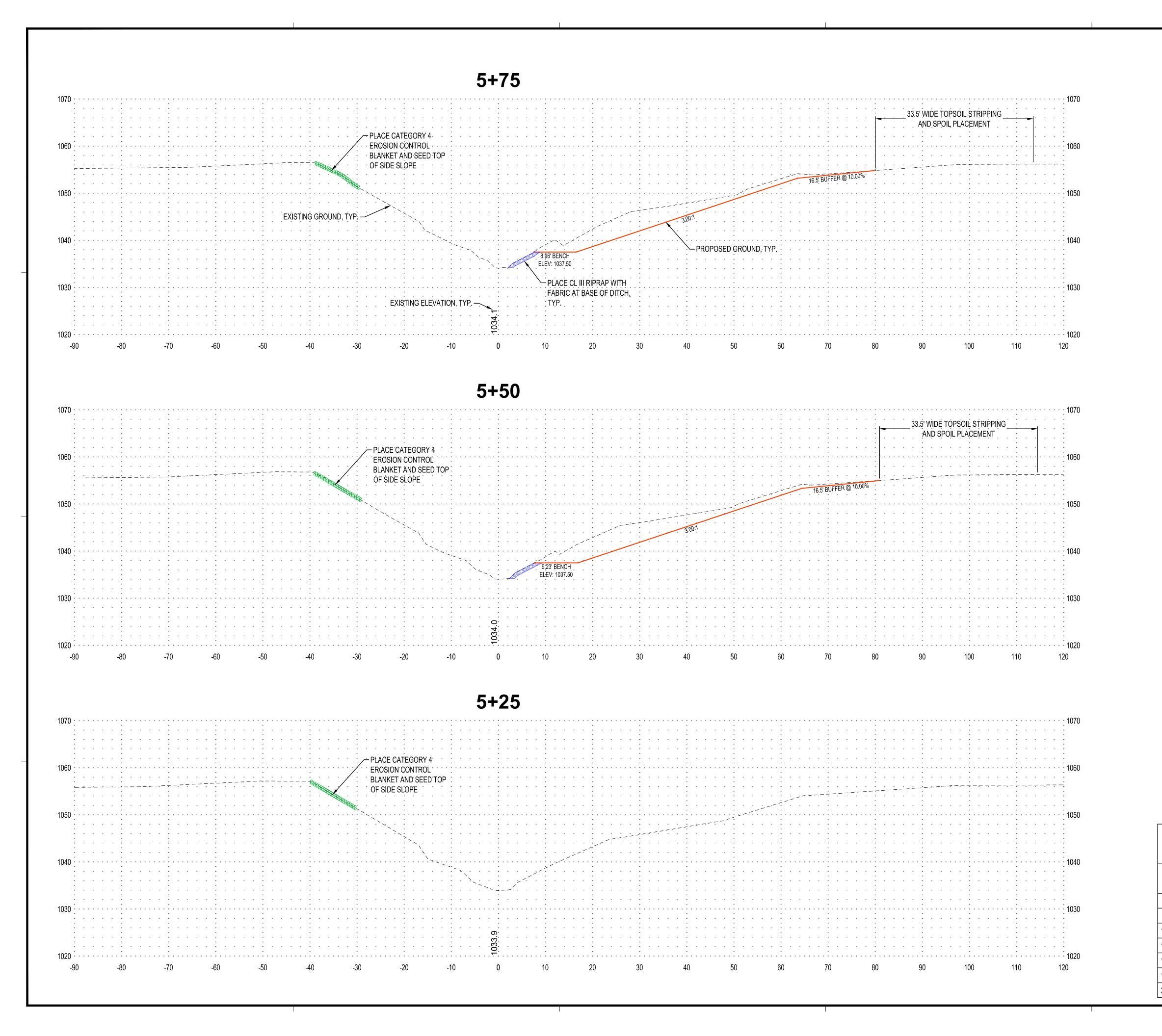
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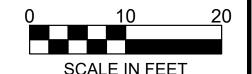
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FARIBAULT

DITCH 24 REPAIR

MINNESOTA

FARIBAULT COUNTY **REVISION SCHEDULE** DATE DESCRIPTION PROJECT NO. 19-23499 FILE NAME 23499 SECTIONS

SMW

MAO

SLOUGH REPAIR I FGFND

CROSS SECTIONS AS VIEWED IN THE DIRECTION OF

23.5' TOPSOIL STRIPPING AND SPOIL PLACEMENT IS

MEASURED FROM 10' OUTSIDE OF 16.5' BUFFER

MEASURED FROM EDGE OF 16.5' BUFFER

33.5' TOPSOIL STRIPPING AND SPOIL PLACEMENT

INCREASING STATIONING.

	LLGLIID			
STA. TO STA.	SLOUGH OVERVIEW	LOCATION		
5+30 TO 6+75	SLOUGH #1	NORTH		
8+00 TO 10+15	SLOUGH #2	NORTH		
11+00 TO 11+95	SLOUGH #3	SOUTH		
14+50 TO 15+45	SLOUGH #4	SOUTH		
15+75 TO 16+25	SLOUGH #5	SOUTH		
18+00 TO 20+45	SLOUGH #6	NORTH		
20+30 TO 20+70	SLOUGH #7	SOUTH		

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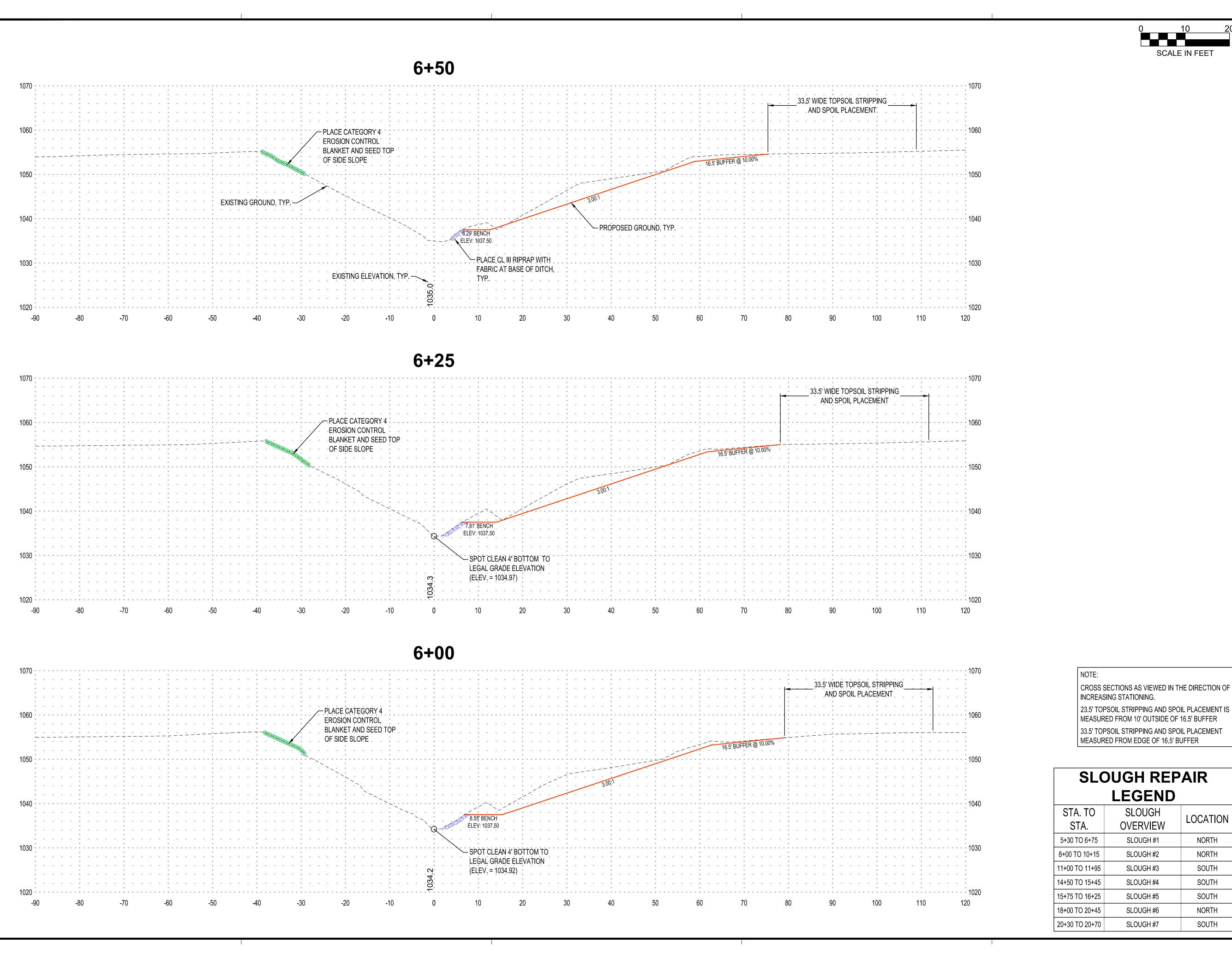
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LEGEND			
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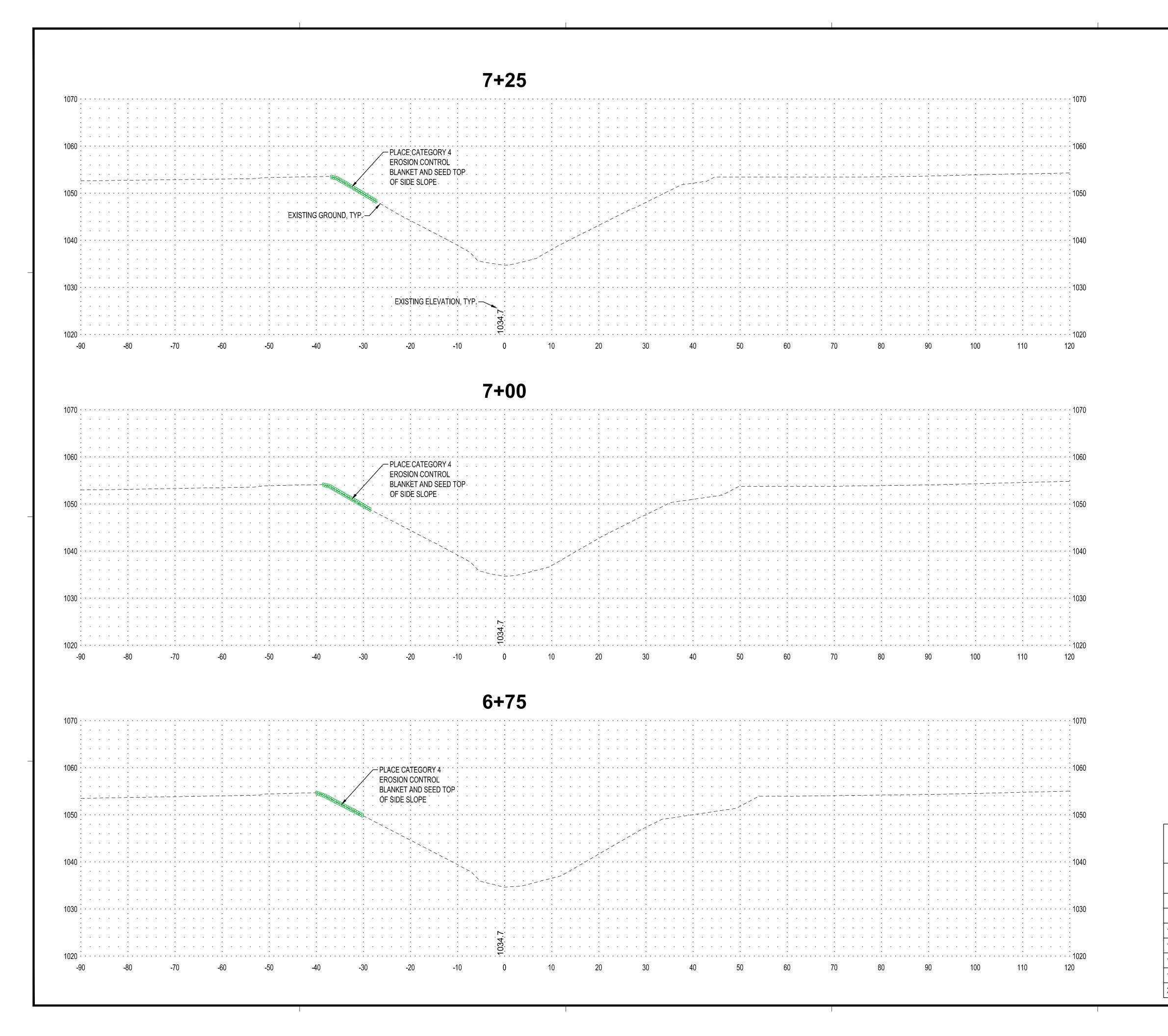
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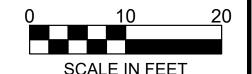
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DITCH 24 REPAIR

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SLOUGH REPAIR LEGEND

CROSS SECTIONS AS VIEWED IN THE DIRECTION OF

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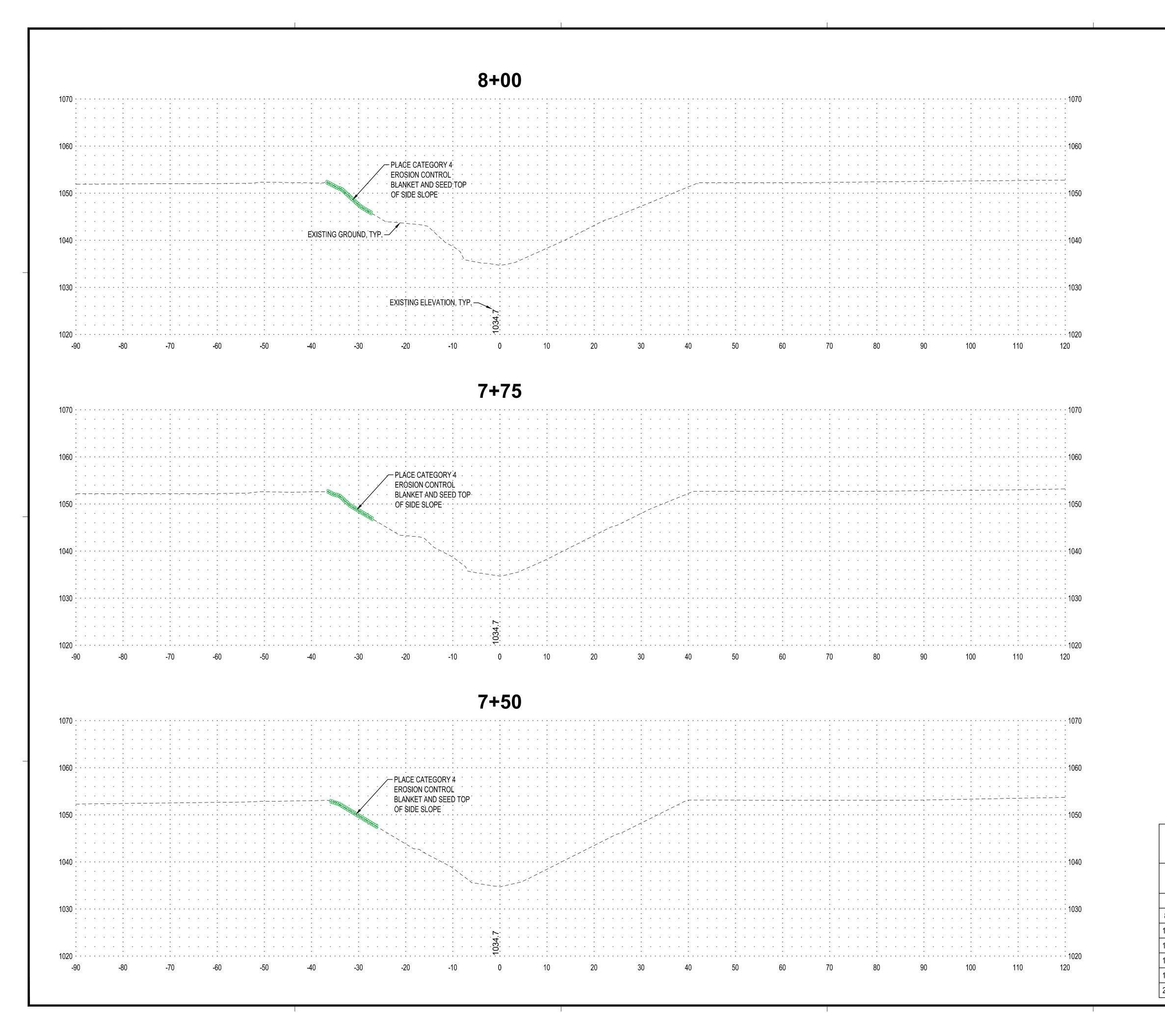
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CROSS-SECTIONS

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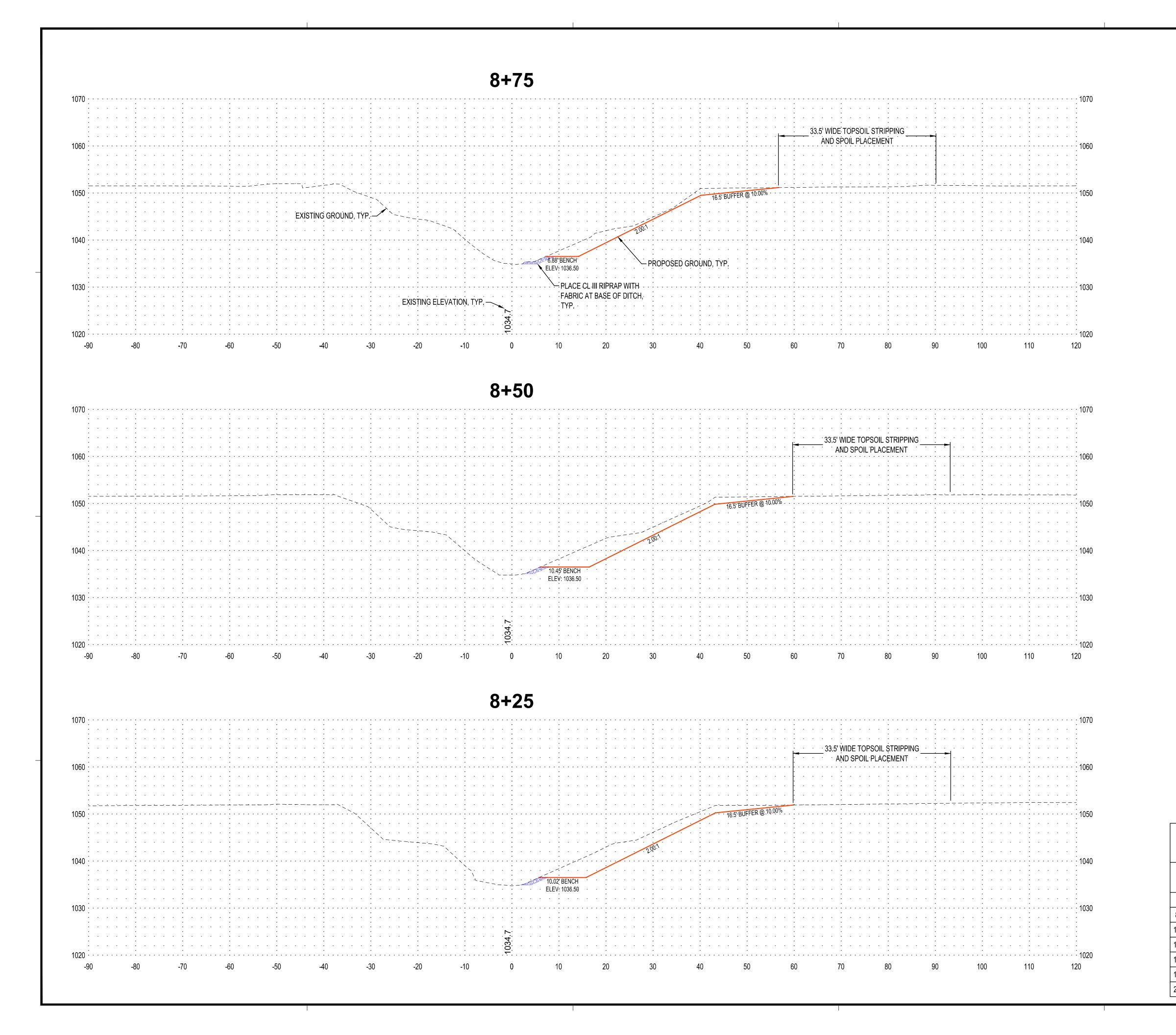
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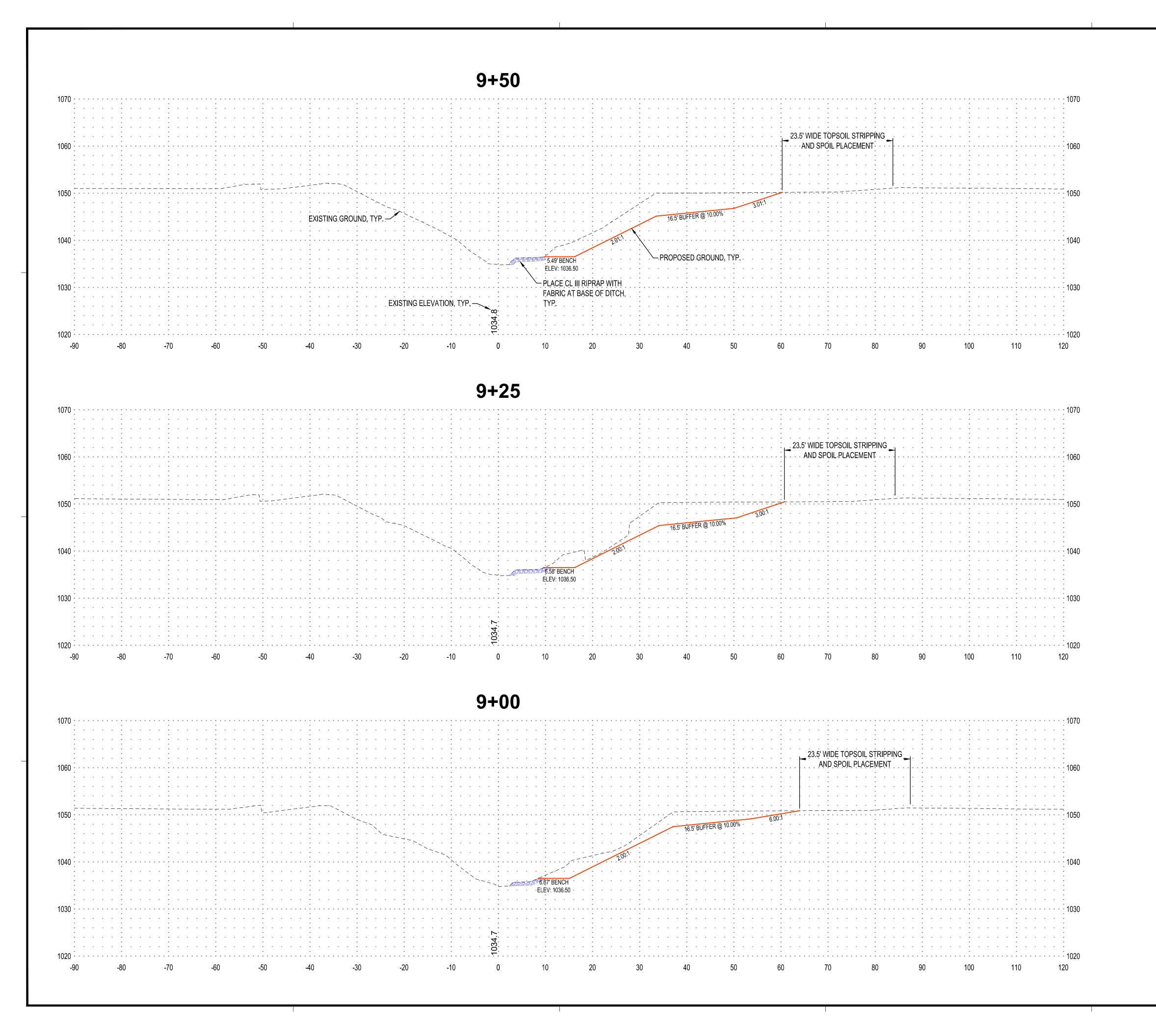
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DITCH 24 REPAIR

MINNESOTA

FARIBAULT COUNTY

PROJECT NO. 19-23499

SMW

MAO

23499 SECTIONS

SLOUGH REPAIR LEGEND

MEASURED FROM EDGE OF 16.5' BUFFER

CROSS SECTIONS AS VIEWED IN THE DIRECTION OF

23.5' TOPSOIL STRIPPING AND SPOIL PLACEMENT IS

MEASURED FROM 10' OUTSIDE OF 16.5' BUFFER

33.5' TOPSOIL STRIPPING AND SPOIL PLACEMENT

INCREASING STATIONING.

STA. TO STA.	SLOUGH OVERVIEW	LOCATION
5+30 TO 6+75	SLOUGH #1	NORTH
8+00 TO 10+15	SLOUGH #2	NORTH
11+00 TO 11+95	SLOUGH #3	SOUTH
14+50 TO 15+45	SLOUGH #4	SOUTH
15+75 TO 16+25	SLOUGH #5	SOUTH
18+00 TO 20+45	SLOUGH #6	NORTH
20+30 TO 20+70	SLOUGH #7	SOUTH

TITLE

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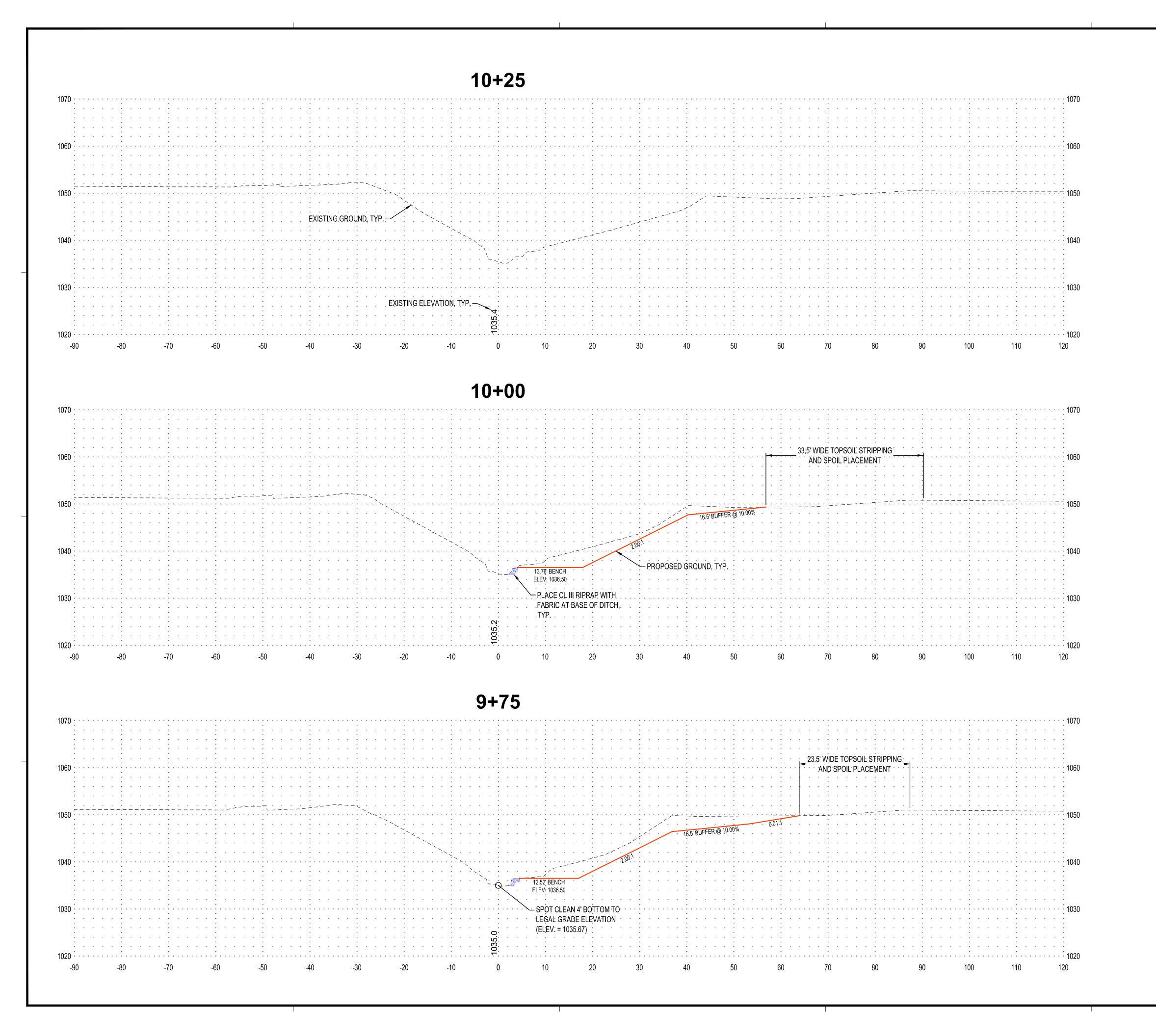
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PROJECT

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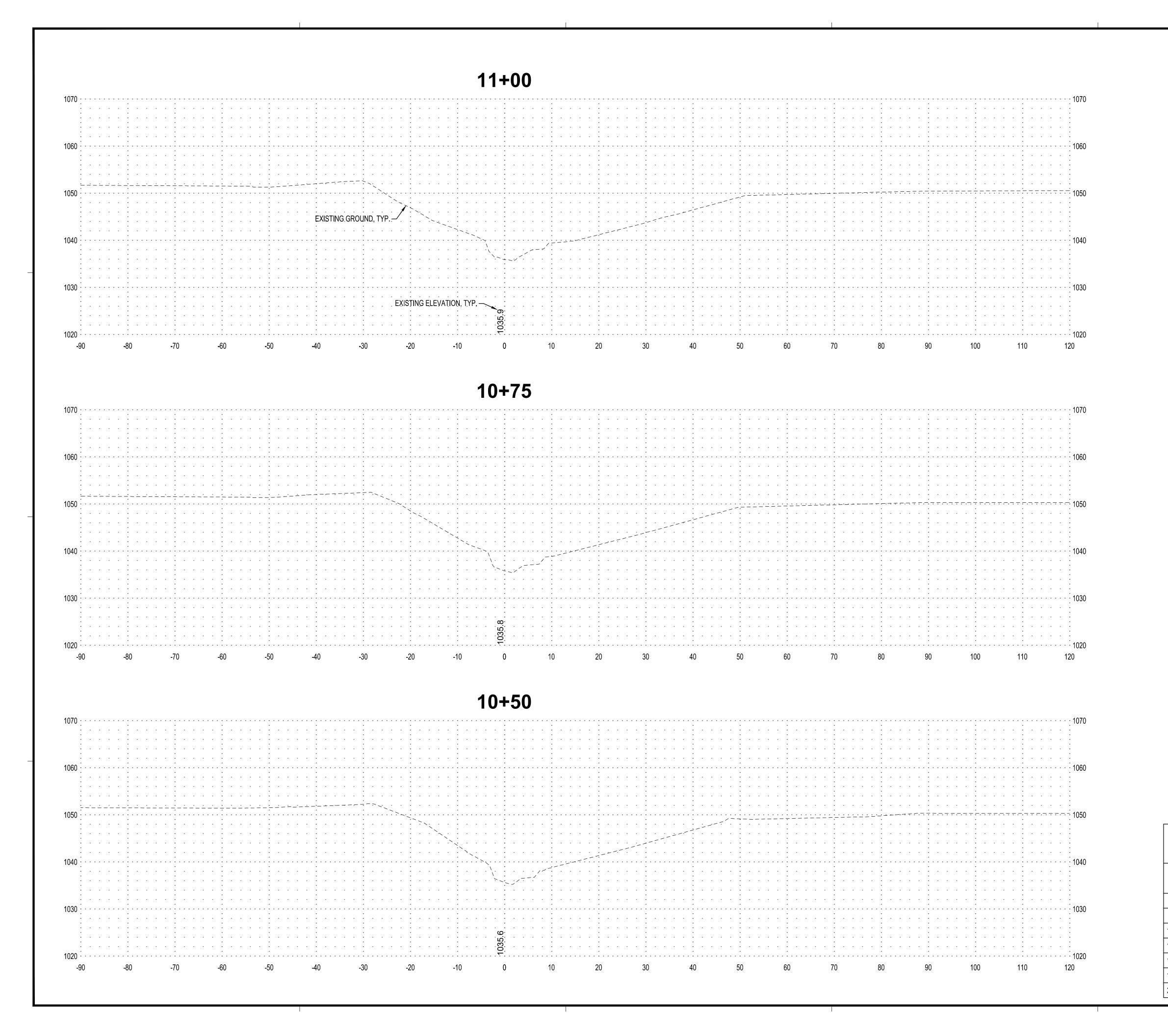
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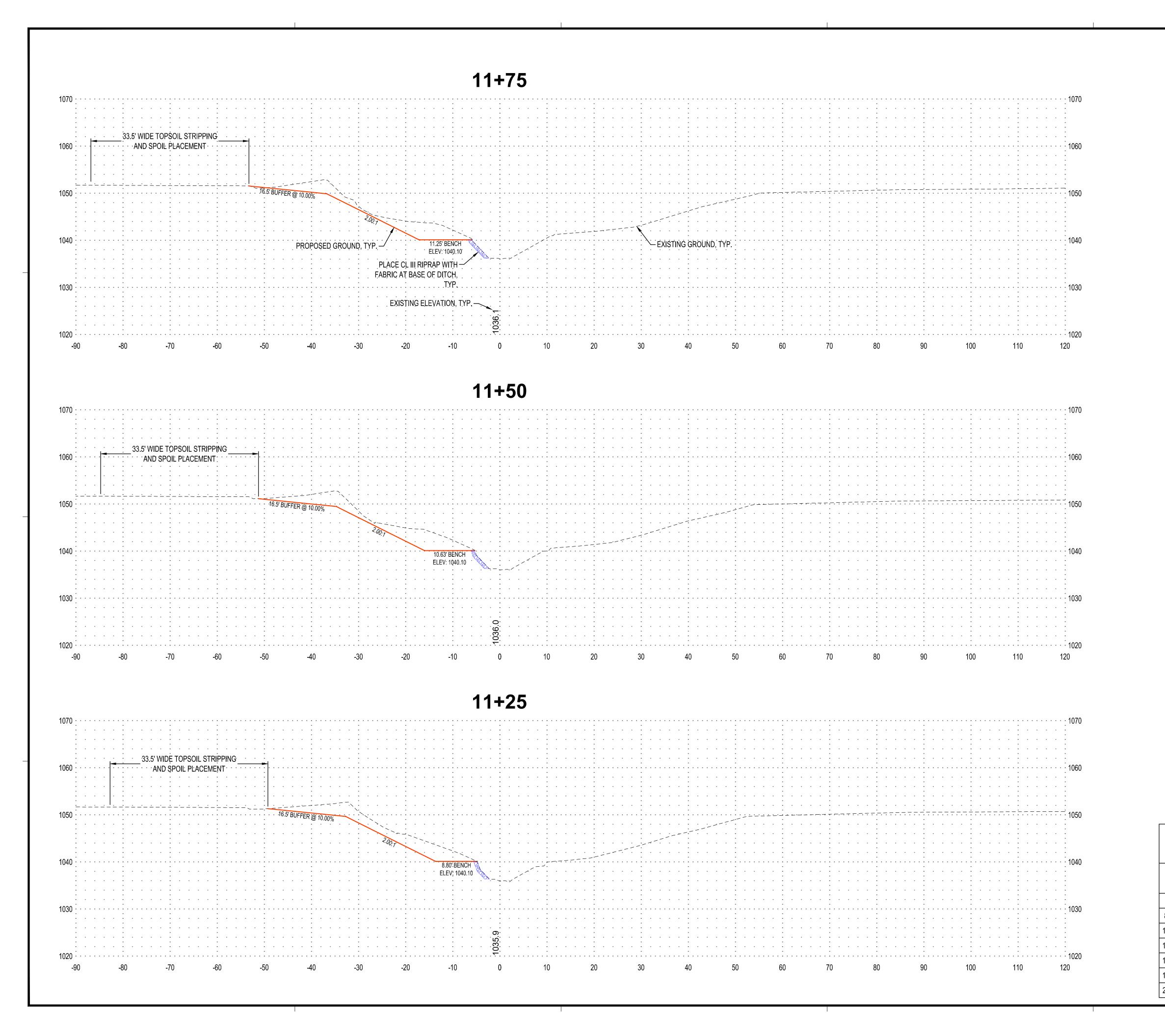
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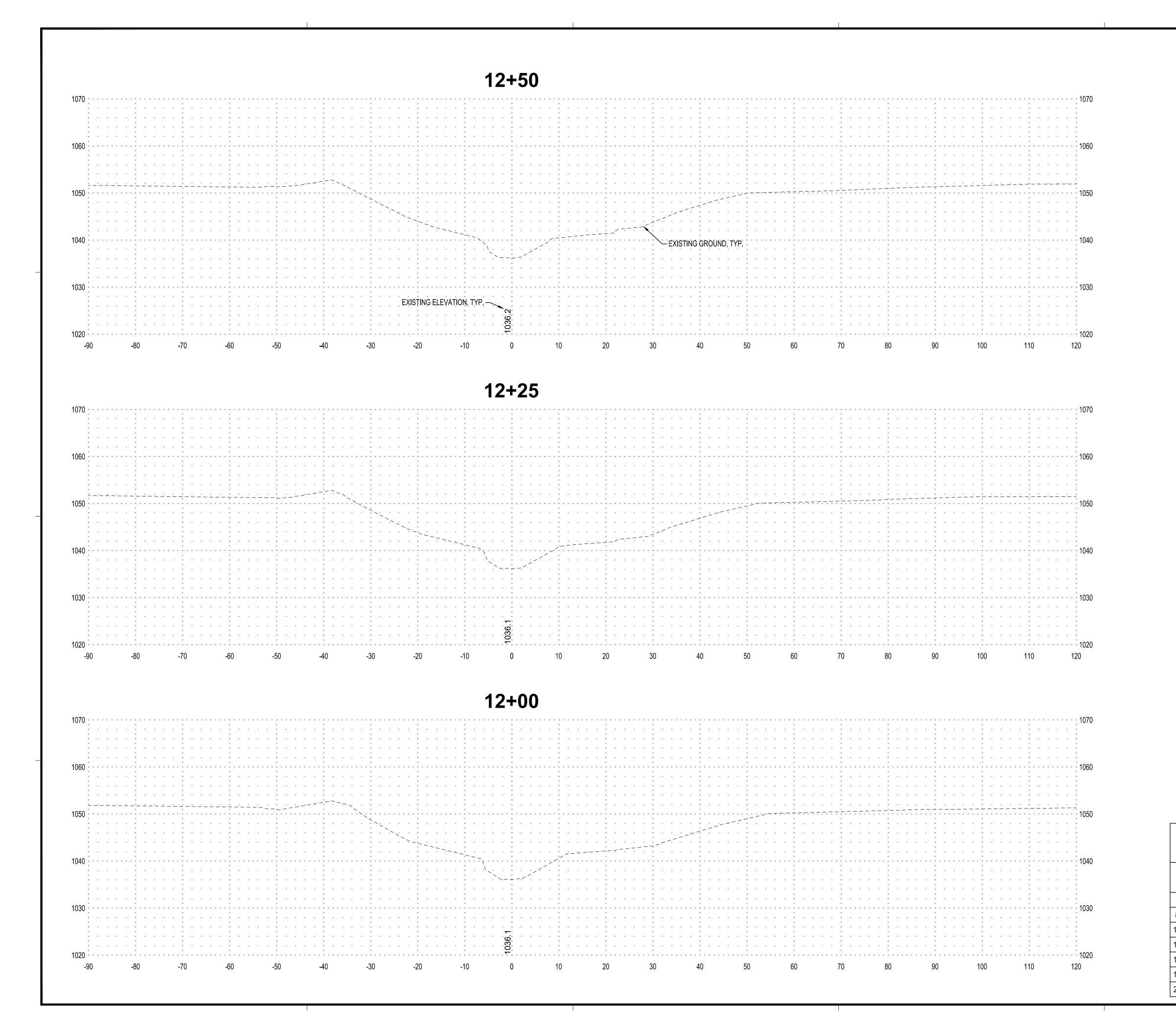
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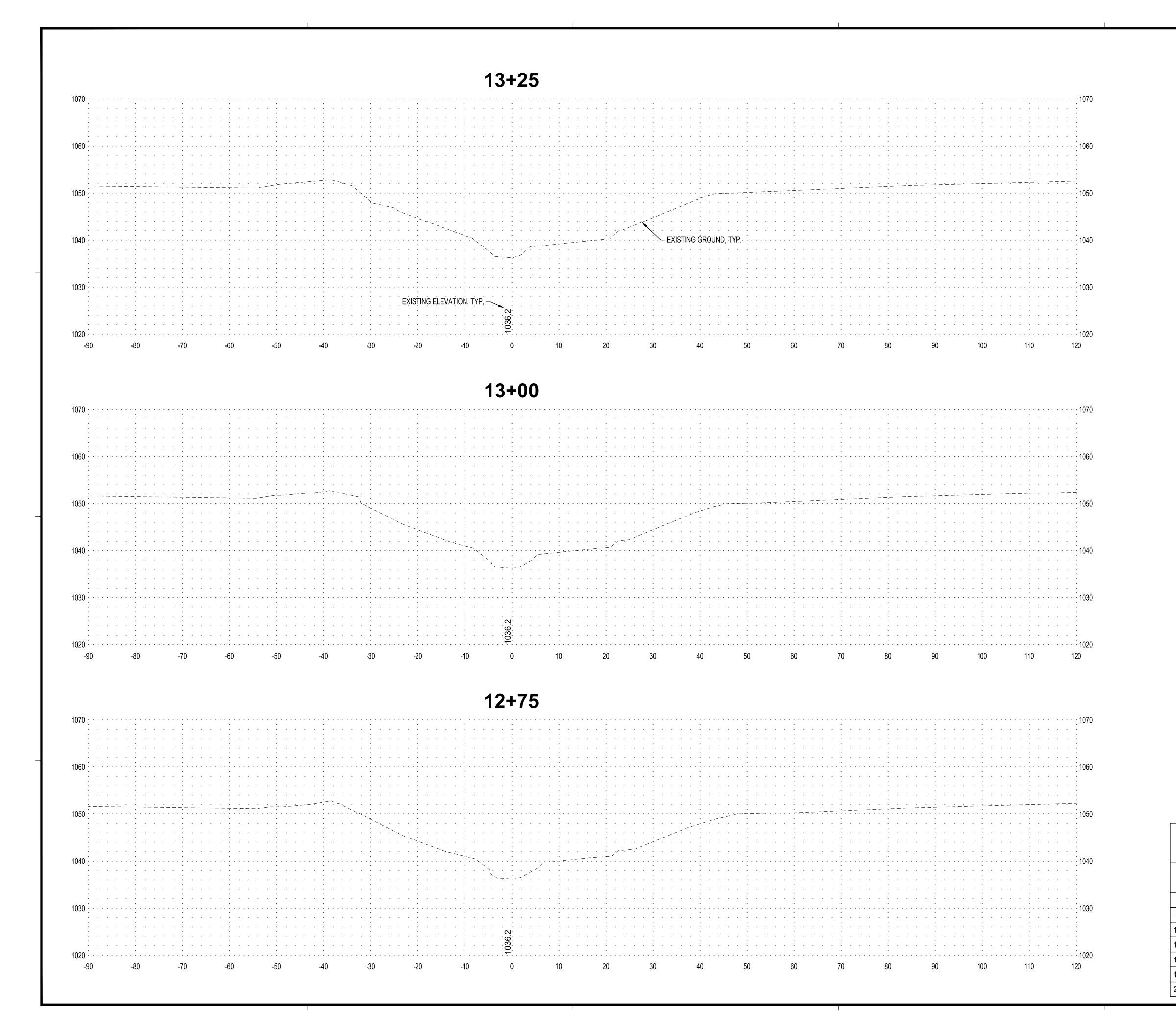
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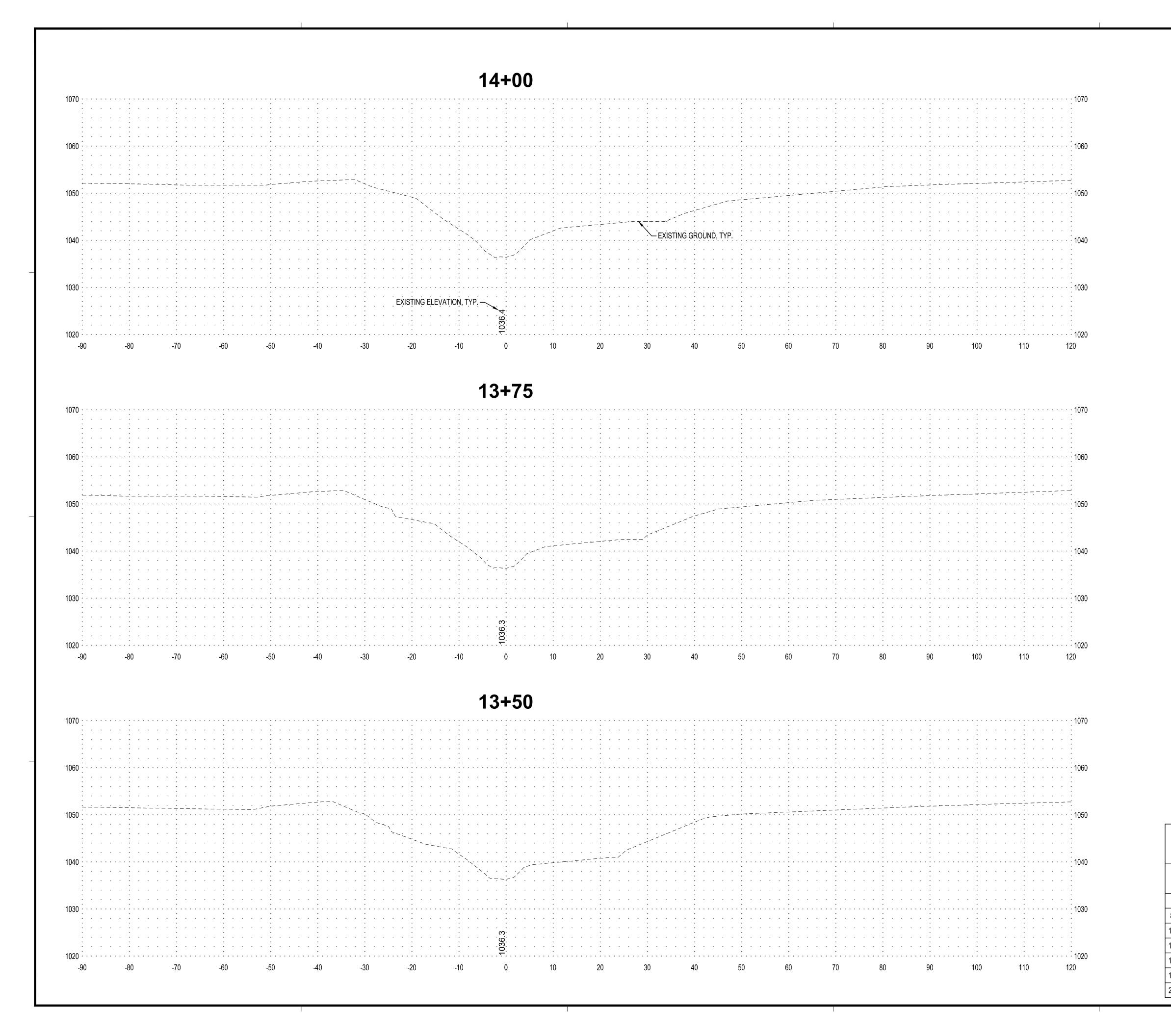
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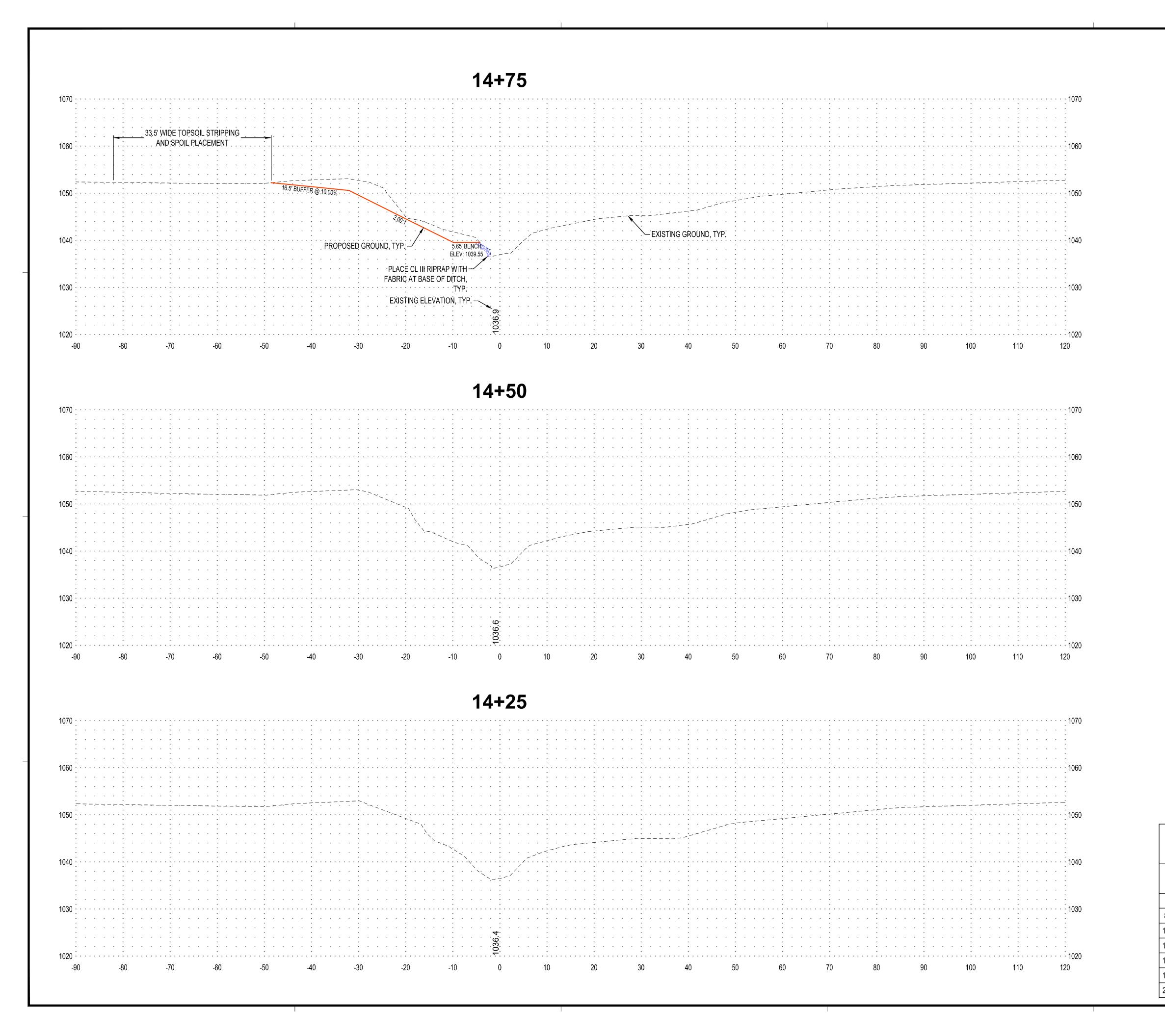
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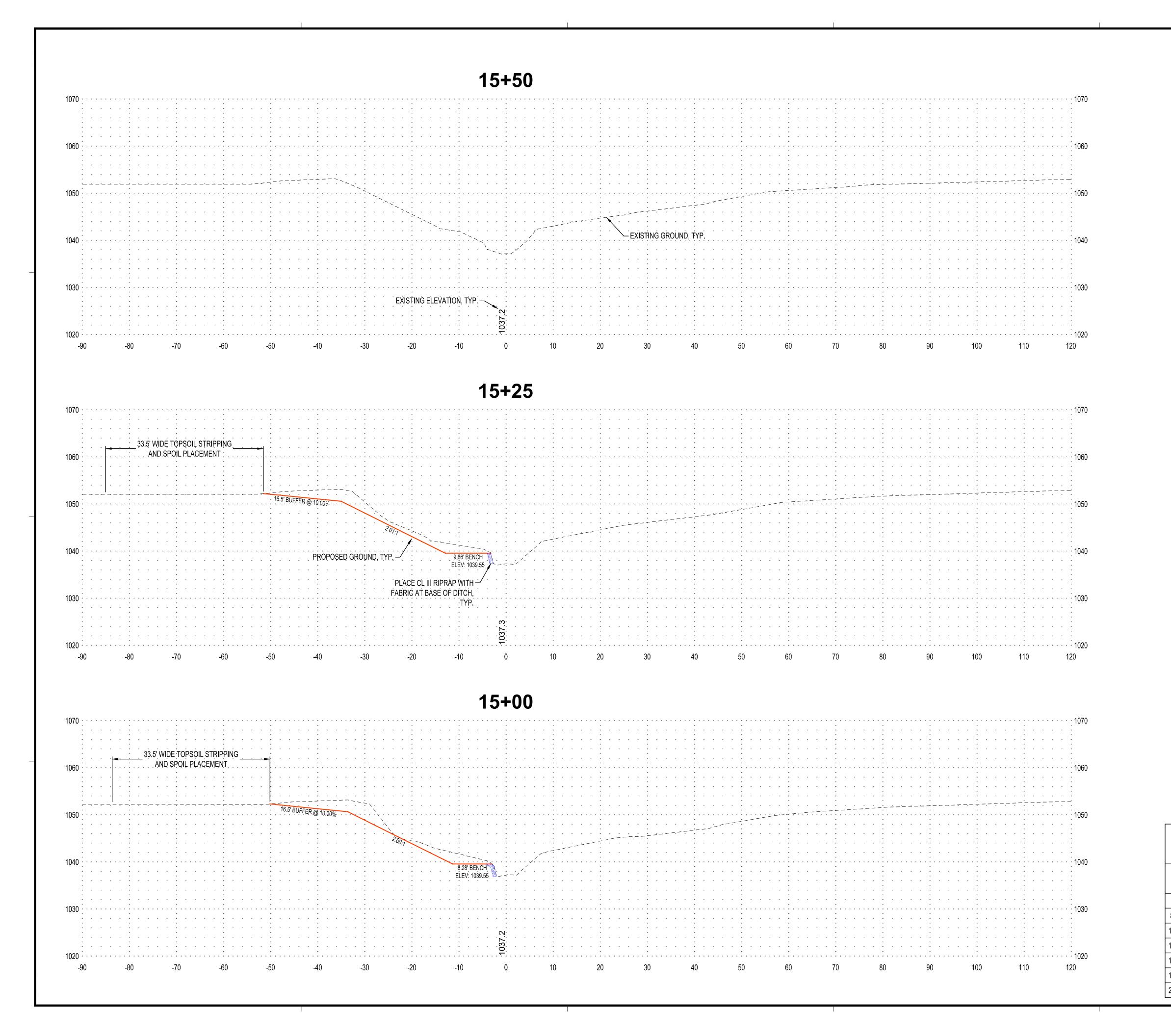
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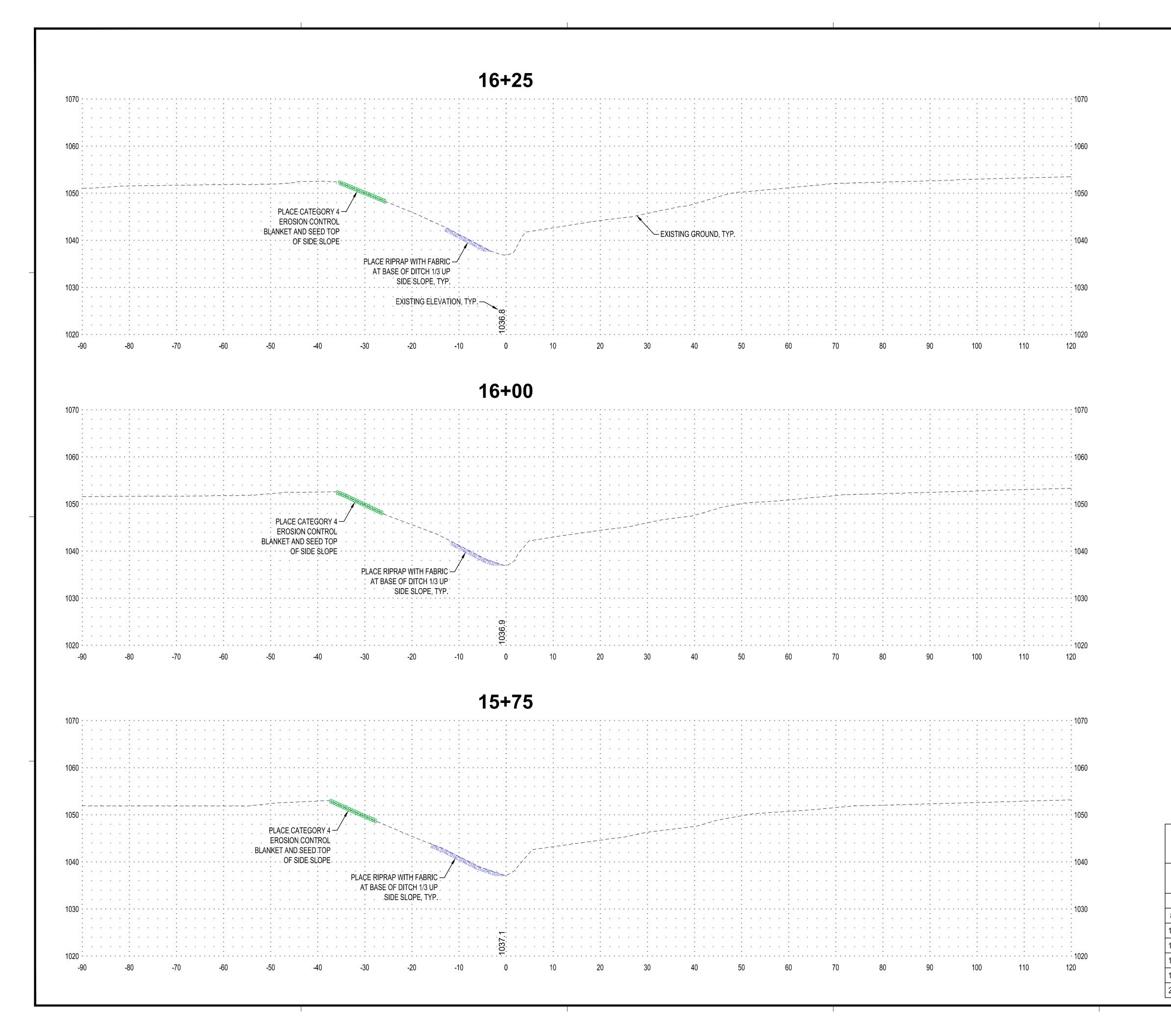
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APPENDIX B: REPAIR PETITION



PETITION FOR REPAIR OF DRAINAGE SYSTEM

Drainage System:Location of System:

County Ditch # 24 Winnebago City 9

Section

SEAVE, NEA

BEFORE THE FARIBAULT COUNTY BOARD OF COMMISSIONERS ACTING AS THE DRAINAGE AUTHORITY FOR FARIBAULT COUNTY DITCH #_ ユギ

WHEREAS, Petition(s) are individuals or an entity interested in or affected by Faribault County Ditch # 2 #; and

WHEREAS, Faribault County Ditch # 24 is in need of repair; and

WHEREAS, Petitioner(s) are requesting that the following locations on Faribault County Ditch #24 are in need of repair; and

WHEREAS, Petitioners believe this drainage system needs stop the erosion
Repair Sluff areas, Need another Drop In Let, & Raise
extisting out lets on South Side fix driveway entrances,
More riprap on Control Structure, Area needs reseded, Straighten
WHEREAS, Petitioner(s) requests that Repair Inspection be done by the selected option: Under Rd

Without an Engineer being appointed by the Drainage Authority and allows for a full drainage inspection to be done and to the hiring of a contractor to do a preliminary investigation.

Or

With the Faribault County Board of Commissioners acting as Drainage Authority for the Faribault County Ditch identified and to proceed in the repair procedure under Minn. Stat. § 103E.715, Subd. 2. This requires appointing an Engineer and holding a public hearing to examine the drainage system.

MINNESOTA STATUTE 103E.701 DEFINES A REPAIR AS:

The term "repair," as used in this section, means to restore all or a part of a drainage system as nearly as practicable to the same condition as originally constructed and subsequently improved, including re-sloping of ditches and leveling of waste banks if necessary to prevent further deterioration, realignment to original construction if necessary to restore the effectiveness of the drainage system, and routine operations that may be required to remove obstructions and maintain the efficiency of the drainage system.

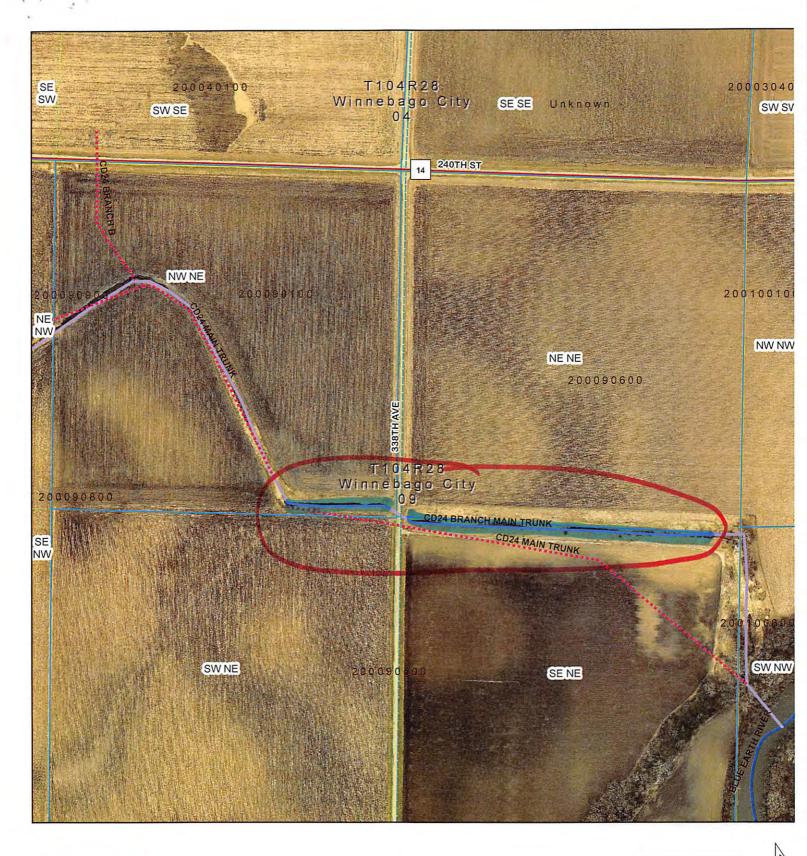
"Repair" also includes:

- Incidental straightening of a tile system resulting from the tile-laying technology used to replace tiles;
 and
- 2. Replacement of tiles with the next larger size that is readily available, if the original size is not readily available.

	this project, I have read and t the Petition for Repair of Dr	_	ion that has been
Petitioner Signature: Print Name: Address:	Jynn Cole 1411 Cole 23278-338th Ave	Date: 4 Phone: 5 Winnebago, Ma	1 5 /19 57-248-6107 1 56098
Petitioner Signature: Print Name: Address:		Date:Phone:	
Petitioner Signature: Print Name: Address:		Date: Phone:	
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Records of Repairs? Drainage Maps?	eeding repair is/are attached? 🔽 🗋	F	eribeuit County Auditor Deputy

Page 2 of 2

Faribault County Petition for Repair of a Drainage System (Revised May 2012)

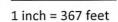




www.faribaultcountyswcd.com

www.co.faribault.mn.us

FARIBAULT COUNTY
SOIL & WATER CONSERVATION DISTRICT
PLANNING & ZONING
DRAINAGE DEPARTMENT



Disclaimer: Faribault County and Faribault Cour SWCD do not warrant or guarantee accuracy the GIS data. The data is meant for reference purposes only and should not be used for offici decisions. The data contained in the maps we compiled from the best available records the could be found and may contain errors or omissio







103E.715 REPAIR BY PETITION.

Subdivision 1. Repair petition. An individual or an entity interested in or affected by a drainage system may file a petition to repair the drainage system. The petition must state that the drainage system needs repair. The auditor shall present the petition to the board at its next meeting or, for a joint county drainage system, to the drainage authority within ten days after the petition is filed.

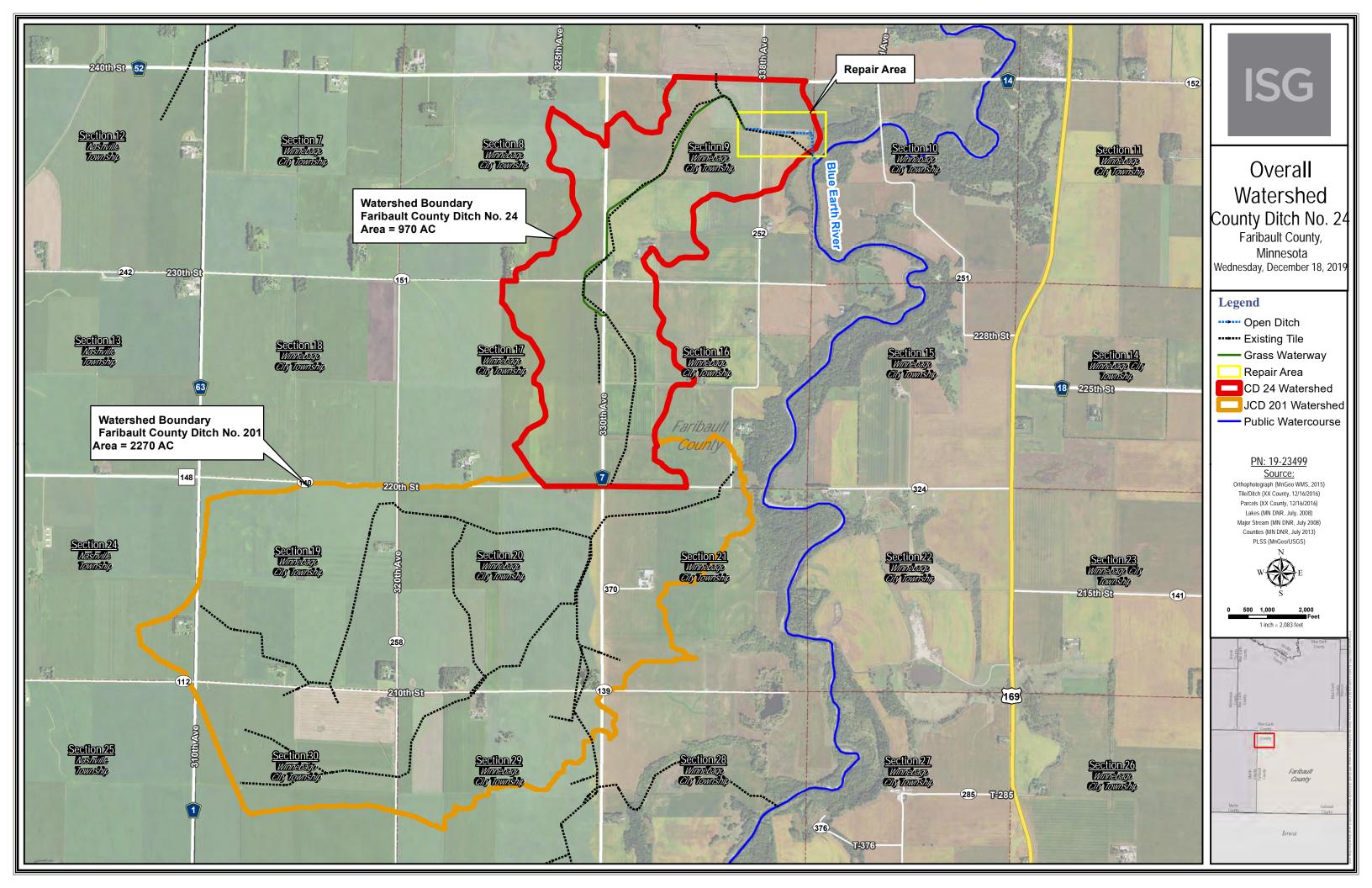
- Subd. 2. Engineer's repair report. If the drainage authority determines that the drainage system needs repair, the drainage authority shall appoint an engineer to examine the drainage system and make a repair report. The report must show the necessary repairs, the estimated cost of the repairs, and all details, plans, and specifications necessary to prepare and award a contract for the repairs. The drainage authority may give notice and order a hearing on the petition before appointing the engineer.
- Subd. 3. Notice of hearing. When the repair report is filed, the auditor shall promptly notify the drainage authority. The drainage authority in consultation with the auditor shall set a time, by order, not more than 30 days after the date of the order for a hearing on the repair report. At least ten days before the hearing, the auditor shall give notice by mail of the time and location of the hearing to the petitioners, owners of property, and political subdivisions likely to be affected by the repair in the repair report.
- Subd. 4. **Hearing on repair report.** (a) The drainage authority shall make findings and order the repair to be made if:
- (1) the drainage authority determines from the repair report and the evidence presented that the repairs recommended are necessary for the best interests of the affected property owners; or
- (2) the repair petition is signed by the owners of at least 26 percent of the property area affected by and assessed for the original construction of the drainage system, and the drainage authority determines that the drainage system is in need of repair so that it no longer serves its original purpose and the cost of the repair will not exceed the total benefits determined in the original drainage system proceeding.
- (b) The order must direct the auditor and the chair of the board or, for a joint county drainage system, the auditors of the affected counties to proceed and prepare and award a contract for the repair of the drainage system. The contract must be for the repair described in the repair report and as determined necessary by the drainage authority, and be prepared in the manner provided in this chapter for the original drainage system construction.
- Subd. 5. Apportioning repair cost for joint county drainage system. For the repair of a joint county drainage system, the drainage authority shall, by order, apportion the repair cost among affected counties in the same manner required in the original construction of the drainage system.
- Subd. 6. Repair by resloping ditches, incorporating multistage ditch cross-section, leveling spoil banks, installing erosion control, or removing trees. (a) For a drainage system that is to be repaired by resloping ditches, incorporating a multistage ditch cross-section, leveling spoil banks, installing erosion control measures, or removing trees, before ordering the repair, the drainage authority must appoint viewers to assess and report on damages and benefits if it determines that:
- (1) the resloping, incorporation of a multistage ditch cross-section, spoil bank leveling, installation of erosion control measures, or tree removal will require the taking of any property not contemplated and included in the proceeding for the establishment or subsequent improvement of the drainage system; or
- (2) any spoil bank leveling or tree removal will directly benefit property where the spoil bank leveling or tree removal is specified.

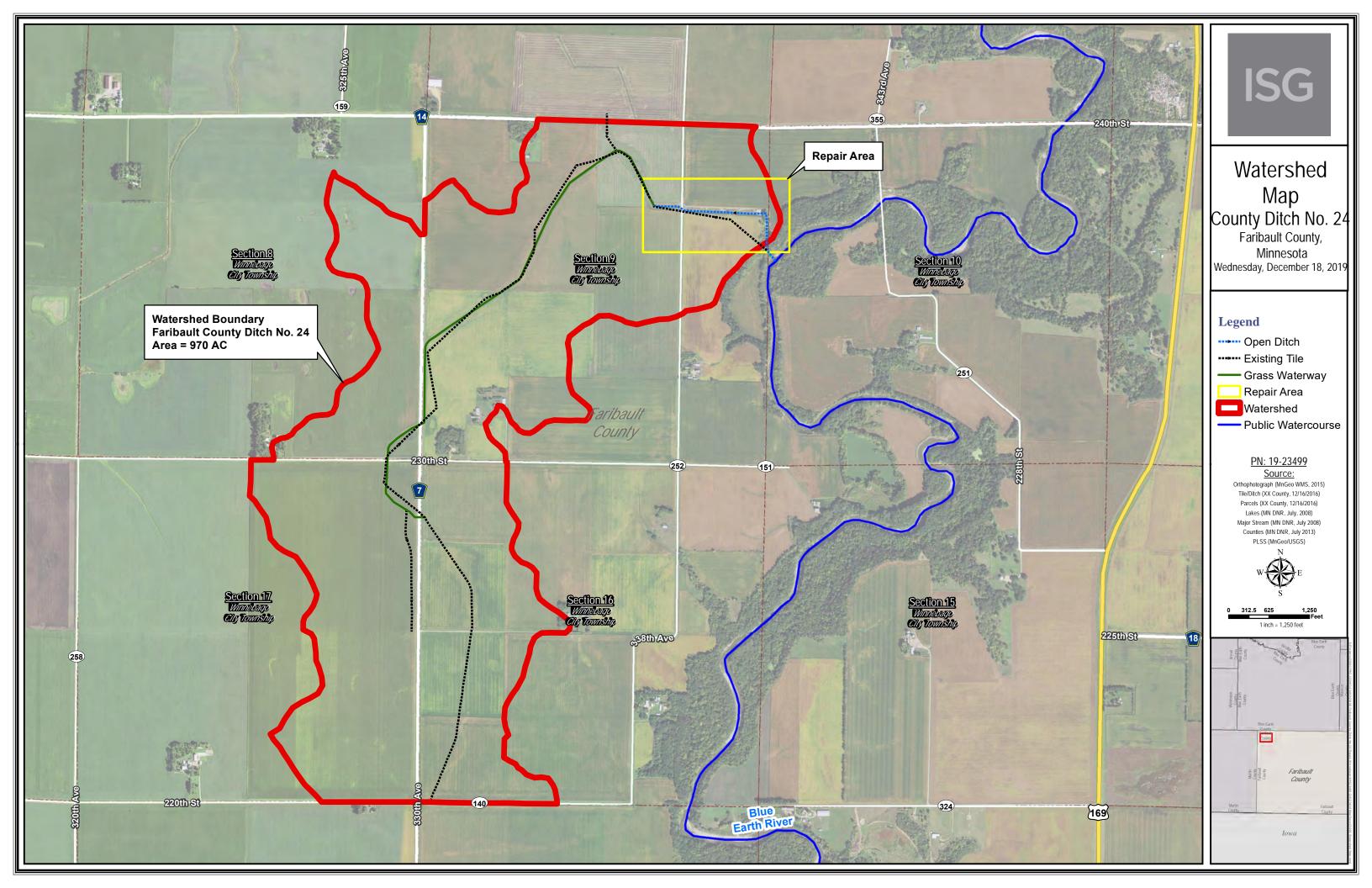
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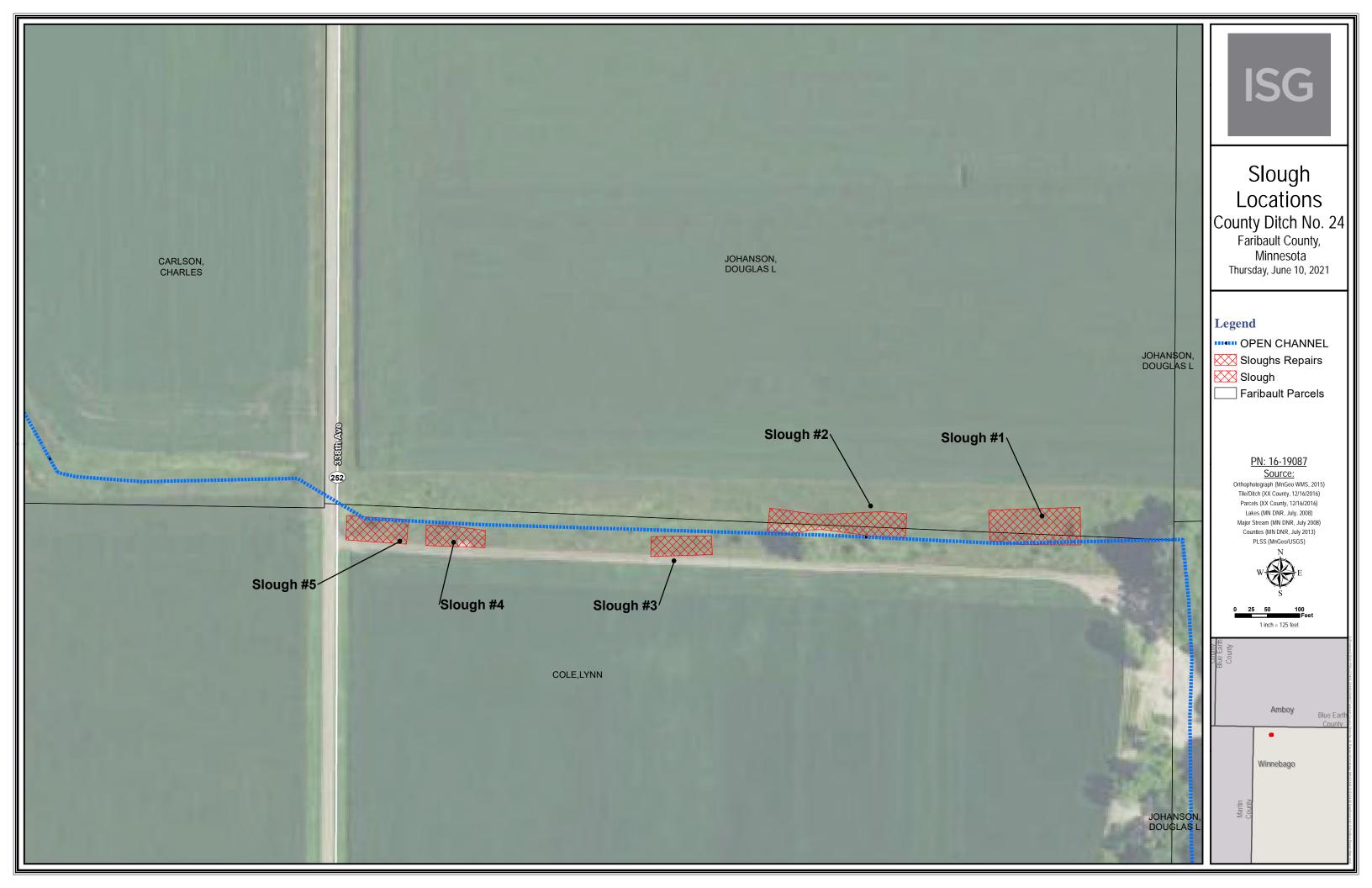
(b) The viewers shall assess and report damages and benefits as provided by sections 103E.315 and 103E.321. The drainage authority shall hear and determine the damages and benefits as provided in sections 103E.325, 103E.335, and 103E.341. The hearing shall be held within 30 days after the property owners' report is mailed. Damages must be paid as provided by section 103E.315 as a part of the cost of the repair, and benefits must be added to the benefits previously determined as the basis for the pro rata assessment for the repair of the drainage system for the repair proceeding only.

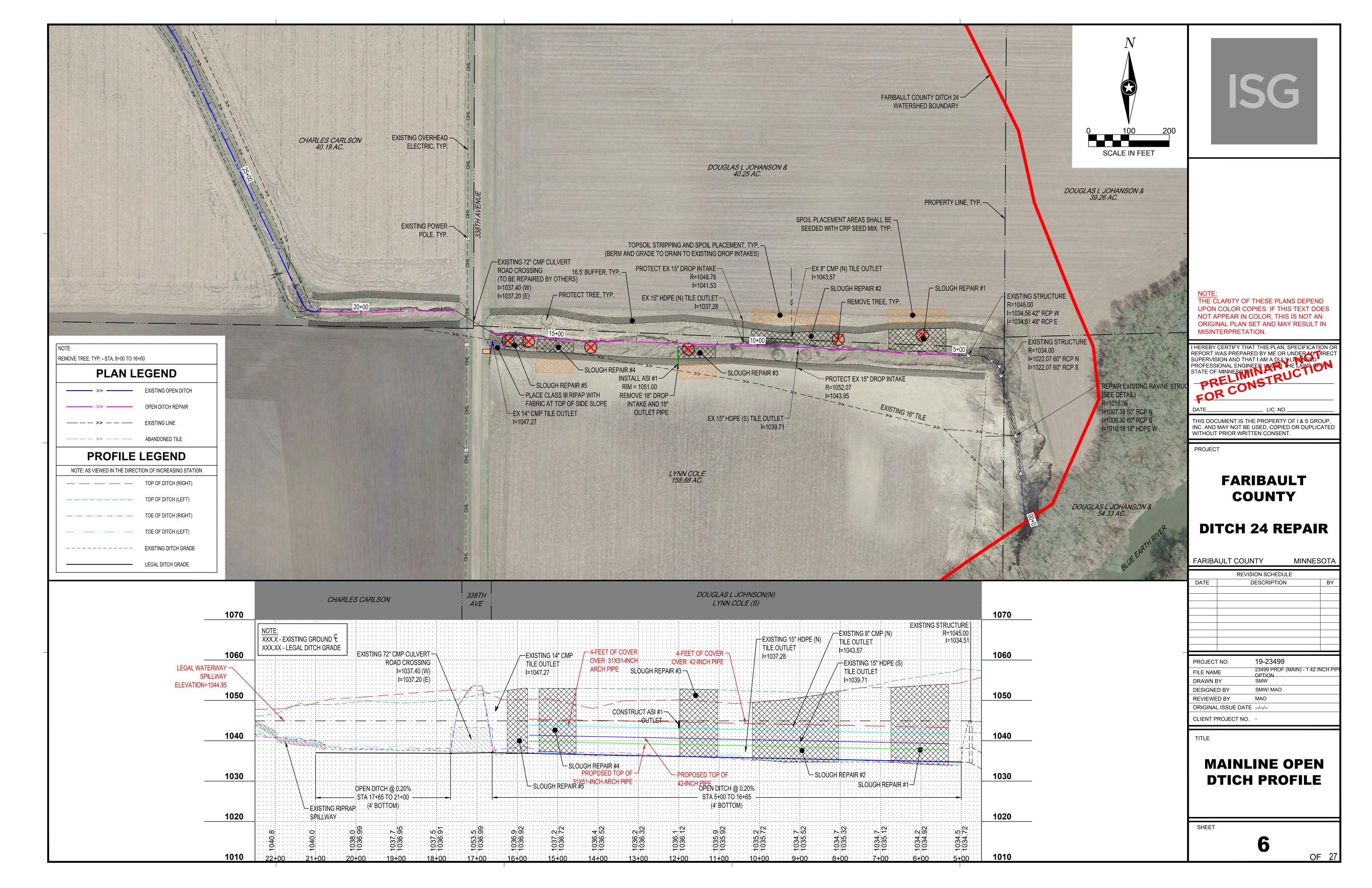
History: 1990 c 391 art 5 s 93; 2013 c 4 s 14

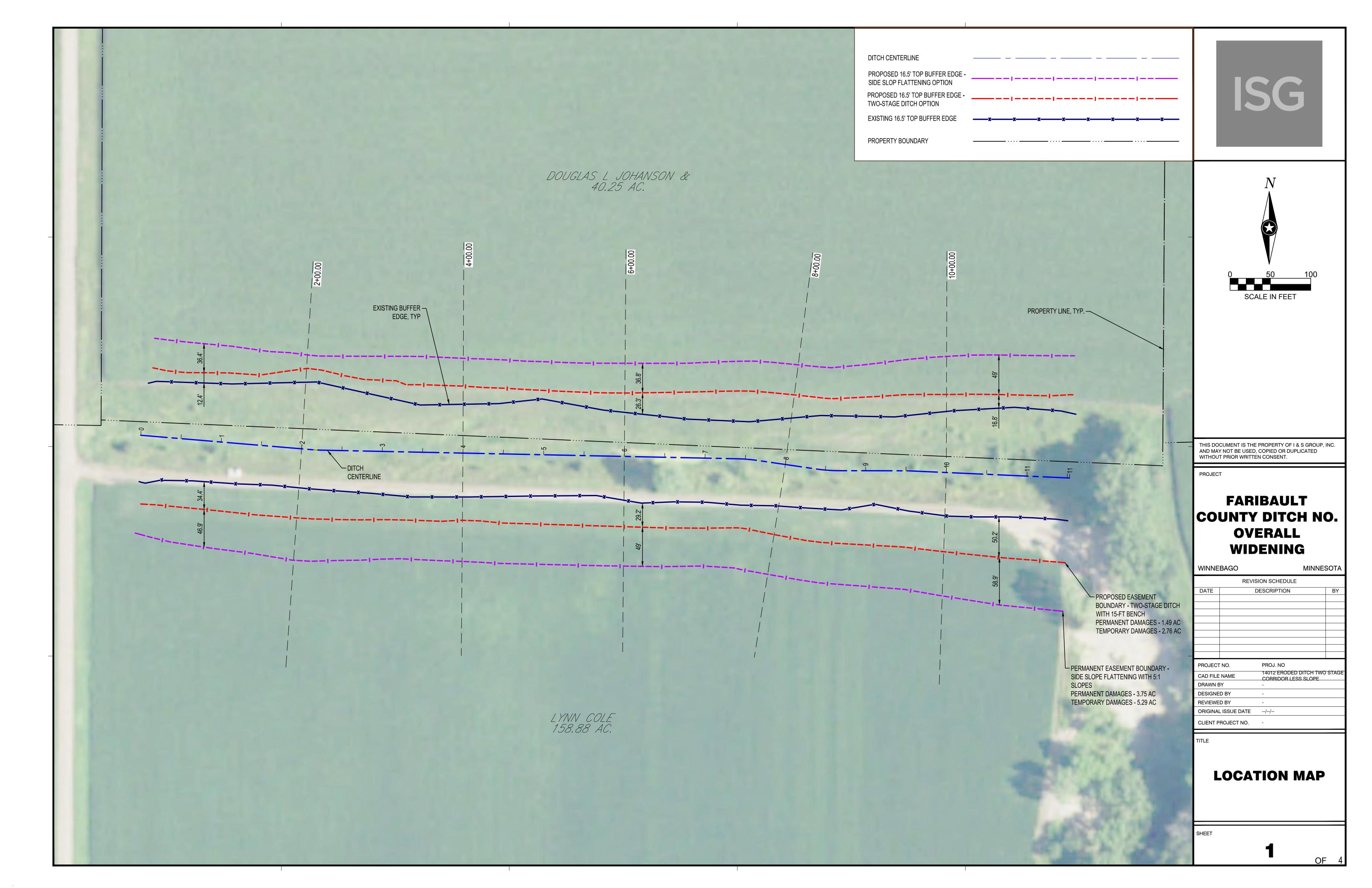
APPENDIX C: MAPS AND EXHIBITS

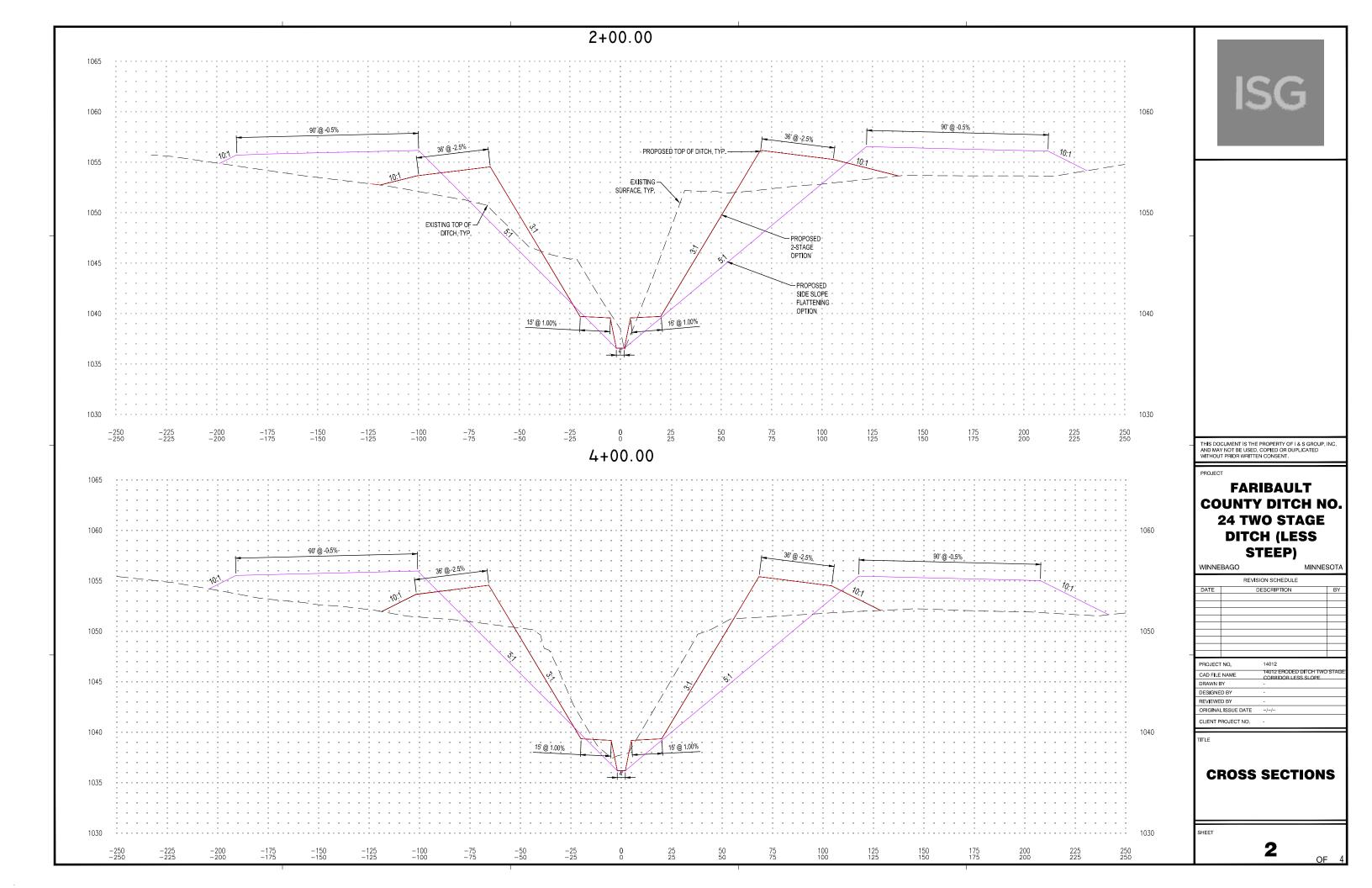


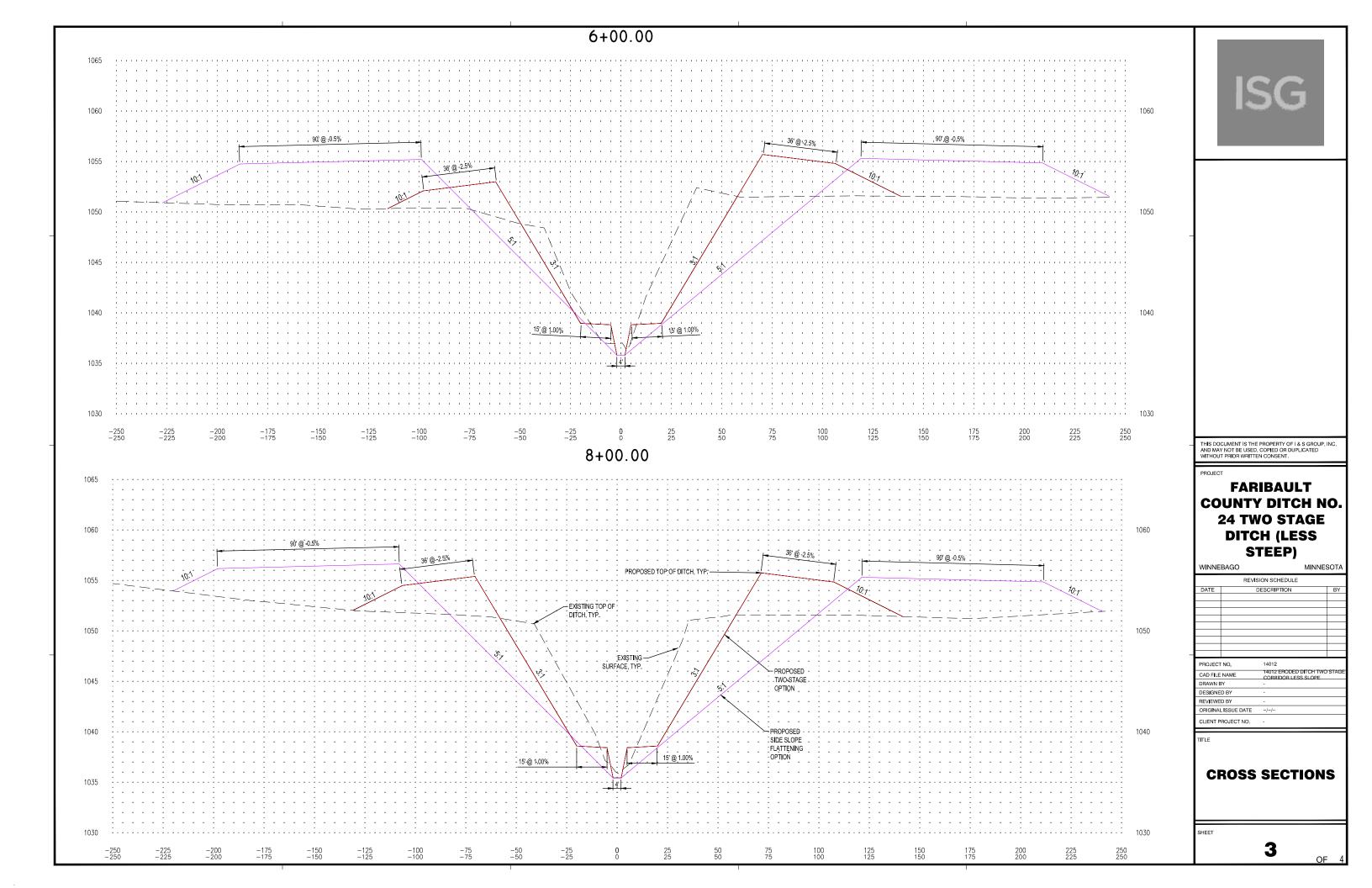


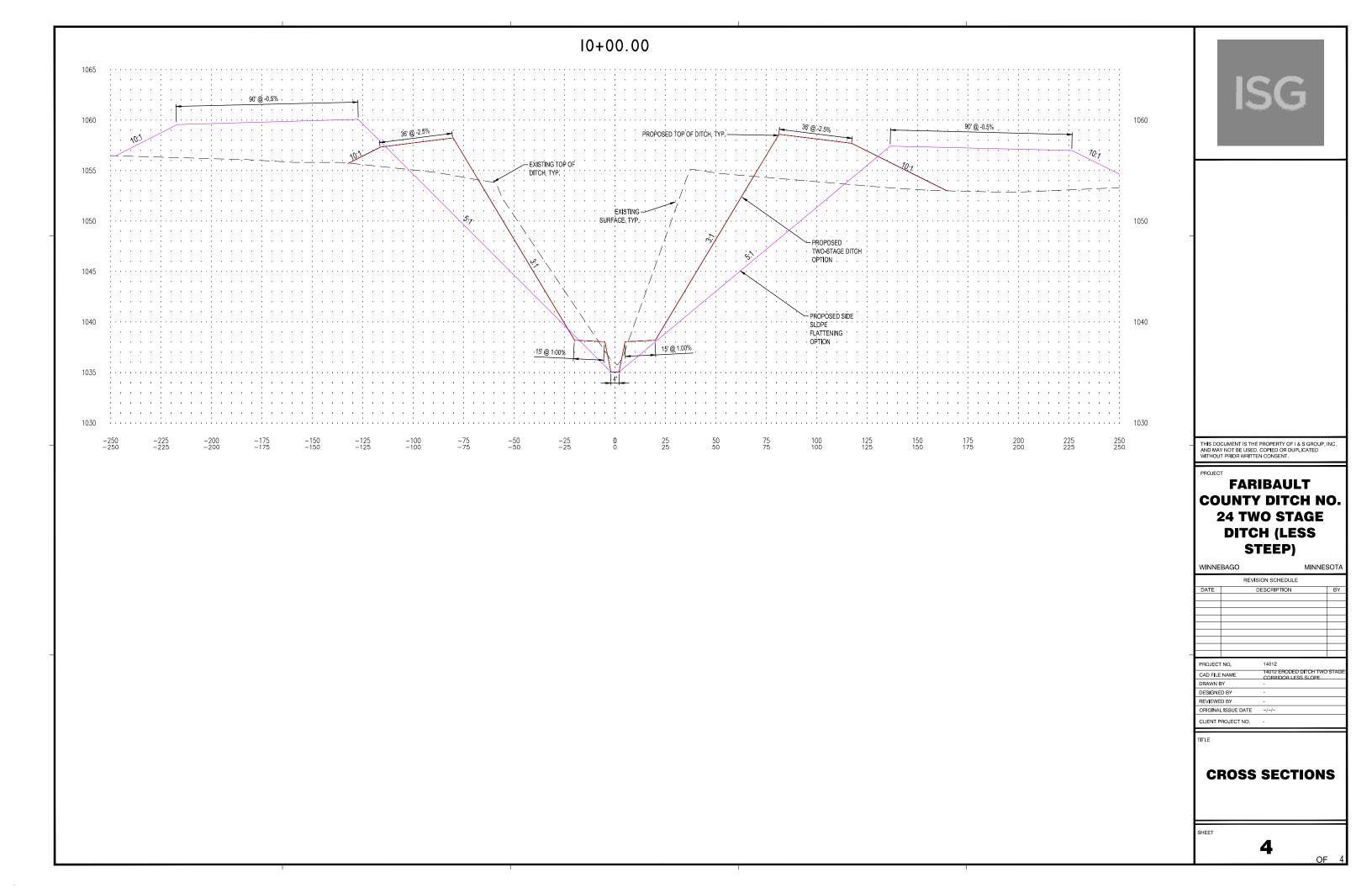












APPENDIX D: COST ESTIMATES

FARIBAULT COUNTY COUNTY DITCH No. 24



COST ESTIMATE SUMMARY

Repair Option	Estimated Cost
PETITIONED REPAIR	\$ 126,105
OPTION 1: 5:1 SIDE SLOPES	\$ 324,591
OPTION 2: TWO-STAGE DITCH	\$ 206,906
OPTION 3: 31" X 51" ARCH PIPE	\$ 431,871
OPTION 3A: 42" RCP PIPE	\$ 345,824
OPTION 3B: 42" DUAL WALL PIPE	\$ 244,086



REPAIR COST ESTIMATE

Slough #1

Item No.	ltem	Unit	Quantity	U	nit Price		Amount
101	MOBILIZATION	LS	1	\$	1,000.00	\$	1,000
102	COMMON EXCAVATION	CY	554	\$	8.00	\$	4,432
103	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	43	\$	80.00	\$	3,437
104	TOP SOIL STRIP & PLACE SPOILS	SY	540	\$	1.00	\$	540
105	16.5' BUFFER STRIP SEEDING	SY	266	\$	3.50	\$	930
105	(SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	01	200	Ф	3.30	Φ	930
	STANDARD SIDESLOPE & BENCH SEEDING						
106	(SEED MIX: BUFFER BLEND WITH Mn DOT CATEGORY 4	SY	856	\$	3.75	\$	3,211
	EROSION CONTROL BLANKET)						
					TOTAL	\$	13,600
	10% UNFORSEEN						1,360
SUBTOTAL						\$	14,960
	TEMPORARY DAMAGES	AC	0.11	\$	650.00	\$	72
		TOTAL	SLOUGH #1	REF	PAIR COST	\$	15,032

Slough #2

Item No.	ltem	Unit	Quantity	U	Init Price		Amount
101	MOBILIZATION	LS	1	\$	1,000.00	\$	1,000
102	COMMON EXCAVATION	CY	789	\$	8.00	\$	6,312
103	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	48	\$	80.00	\$	3,822
104	TOP SOIL STRIP & PLACE SPOILS	SY	800	\$	1.00	\$	800
105	16.5' BUFFER STRIP SEEDING (SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	SY	394	\$	3.50	\$	1,380
106	STANDARD SIDESLOPE & BENCH SEEDING (SEED MIX: BUFFER BLEND WITH Mn DOT CATEGORY 4 EROSION CONTROL BLANKET)	SY	766	\$	3.75	\$	2,871
					TOTAL	\$	16,200
	10% UNFORSEEN						1,620
SUBTOTAL						\$	17,820
	TEMPORARY DAMAGES	AC	0.17	\$	650.00	\$	107
		TOTAL	SLOUGH #2	REF	PAIR COST	\$	17,927

Slough #3

Item No.	Item	Unit	Quantity	U	nit Price	Amount
101	MOBILIZATION	LS	1	\$	1,000.00	\$ 1,000
102	COMMON EXCAVATION	CY	252	\$	8.00	\$ 2,016
103	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	21	\$	80.00	\$ 1,689
104	TOP SOIL STRIP & PLACE SPOILS	SY	354	\$	1.00	\$ 354
105	16.5' BUFFER STRIP SEEDING (SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	SY	174	\$	3.50	\$ 610
106	STANDARD SIDESLOPE & BENCH SEEDING (SEED MIX: BUFFER BLEND WITH Mn DOT CATEGORY 4 EROSION CONTROL BLANKET)	SY	234	\$	3.75	\$ 878
					TOTAL	\$ 6,600
	10% UNFORSEEN					\$ 660
SUBTOTAL						\$ 7,260
	TEMPORARY DAMAGES	AC	0.07	\$	650.00	\$ 47
		TOTAL	SLOUGH#3	REF	PAIR COST	\$ 7,307



REPAIR COST ESTIMATE Slough #4

Item No.	Item	Unit	Quantity	l	Init Price		Amount
101	MOBILIZATION	LS	1	\$	1,000.00	\$	1,000
102	COMMON EXCAVATION	CY	216	\$	8.00	\$	1,728
103	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	32	\$	80.00	\$	2,533
104	INSTALL 24-INCH ASI RISER ASSEMBLY W/TRASH GRATE	EA	1	\$	1,529.70	\$	1,530
105	INSTALL 24-INCH ASI OUTLET ASSEMBLY	EA	1	\$	1,362.10	\$	1,362
106	REMOVE EXISTING SIDE INTAKE	EA	1	\$	250.00	\$	250
107	TOP SOIL STRIP & PLACE SPOILS	SY	354	\$	1.00	\$	354
108	16.5' BUFFER STRIP SEEDING	SY	174	\$	3.50	\$	610
100	(SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	31	174	Φ	3.50	Φ	010
	STANDARD SIDESLOPE & BENCH SEEDING						
109	(SEED MIX: BUFFER BLEND WITH Mn DOT CATEGORY 4	SY	195	\$	3.75	\$	731
	EROSION CONTROL BLANKET)		<u> </u>				
					TOTAL	\$	10,100
10% UNFORSEEN						\$	1,010
SUBTOTAL						\$	11,110
TEMPORARY DAMAGES AC 0.07 \$ 650.00					\$	47	
		TOTAL	SLOUGH #4	REI	PAIR COST	\$	11,157

Slough #5

Item No.	Item	Unit	Quantity	Unit Price			Amount
101	MOBILIZATION	LS	1	\$	1,000.00	\$	1,000
102	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	50	\$	80.00	\$	3,970
103	STANDARD SIDESLOPE & BENCH SEEDING (SEED MIX: BUFFER BLEND WITH Mn DOT CATEGORY 4 EROSION CONTROL BLANKET)	SY	56	\$	3.75	\$	208
_					TOTAL	\$	5,200
10% UNFORSEEN							
SUBTOTAL							
	TEMPORARY DAMAGES	AC	0.00	\$	650.00	\$	-
TOTAL SLOUGH #5 REPAIR COST S							

Tree Removals

Item No.	Item	Unit	Quantity	Į	Jnit Price		Amount
101	MOBILIZATION	LS	1	\$	1,000.00	\$	1,000
102	CLEAR AND GRUB TREE (OVER 6")	EA	6	\$	417.70	\$	2,506
TOTAL							3,600
10% UNFORSEEN						\$	360
				,	SUBTOTAL	\$	3,960

TOTAL REPAIR COST

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Slough #1	\$ 15,032
Slough #2	\$ 17,927
Slough #3	\$ 7,307
Slough #4	\$ 11,157
Slough #5	\$ 5,720
Tree Removals	\$ 3,960

5,500	\$ COUNTY ADMINISTRATION COSTS
9,500	\$ TOPOGRAPHIC SURVEY
26,500	\$ REPORTS, PLANS AND SPECIFICATIONS
23,500	\$ CONSTRUCTION STAKING AND ADMINISTRATION
126.105	\$ COMPLETE REPAIR COST



Proposed 5:1 Side Slope Flattening

5:1 FLATTENED SIDE SLOPES

Item No.	ltem	Unit	Quantity	J	Init Price		Amount	
101	MOBILIZATION	LS	1	\$	10,000.00	\$	10,000	
102	COMMON EXCAVATION	CY	35027	\$	3.00	\$	105,081	
103	TOP SOIL STRIP & PLACE SPOILS	AC	9.0	\$	5,250.00	\$	47,457	
104	16.5' BUFFER STRIP SEEDING	AC	0.9	\$	3,450.00	\$	3,040	
104	(SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	ζ	0.9	9	3,430.00	9	3,040	
105	STANDARD SIDESLOPE SEEDING	AC	3.7	\$	3,450.40	\$	12,770	
105	(SEED MIX: BUFFER BLEND WITH TYPE 8 MULCH)	ζ)	3.1	Ф	3,450.40	9	12,770	
106	INSTALL 24-INCH ASI RISER ASSEMBLY W/TRASH GRATE	EA	6	\$	1,529.70	\$	9,180	
107	INSTALL 24-INCH ASI OUTLET ASSEMBLY	EA	6	\$	1,362.10	\$	8,180	
108	TREE REMOVALS	LS	1	\$	3,960.00	\$	3,960	
					TOTAL		199,668	
			10%	6 UN	IFORSEEN	\$	19,970	
				;	SUBTOTAL	\$	219,638	
	TEMPORARY DAMAGES	AC	5.29	\$	650.00	\$	3,450	
	LAND ACQUISTION/ PERMANENT DAMAGES	AC	3.75	\$	7,500.00	\$	28,100	
		COUNT	ADMINISTF	RATI	ON COSTS	\$	10,982 2,100	
TOPOGRAPHIC SURVEY								
	REPORTS, PLANS AND SPECIFICATIONS							
	CONSTRU	CTION ST	AKING & AD	MIN	ISTRATION	\$	33,053	
	TOTAL 5:1 FLAT	TENED S	IDE SLOPES	RE	PAIR COST	\$	324,591	



Proposed Two Stage Ditch Option (15-ft Bench)

TWO-STAGE DITCH

Item No.	Item	Unit	Quantity	Į	Jnit Price		Amount		
101	MOBILIZATION	LS	1	\$	10,000.00	\$	10,000		
102	COMMON EXCAVATION	CY	16796	\$	3.00	\$	50,387		
103	TOP SOIL STRIP & PLACE SPOILS	AC	4.25	\$	5,250.00	\$	22,313		
104	16.5' BUFFER STRIP SEEDING (SEED MIX: BUFFER BLEND WITH TYPE 3 MULCH)	AC	0.88	\$	3,450.00	\$	3,040		
105	STANDARD SIDESLOPE SEEDING (SEED MIX: BUFFER BLEND WITH TYPE 8 MULCH)	AC	3.9	\$	3,450.40	\$	13,430		
106	INSTALL 24-INCH ASI RISER ASSEMBLY W/TRASH GRATE	EA	6	\$	1,529.70	\$	9,180		
107	INSTALL 24-INCH ASI OUTLET ASSEMBLY	EA	6	\$	1,362.10	\$	8,180		
108	TREE REMOVALS	LS	1	\$	3,960.00	\$	3,960		
					TOTAL		120,489		
			10%	/U %	NFORSEEN	\$	12,050		
					SUBTOTAL	\$	132,539		
	TEMPORARY DAMAGES	AC	2.76	\$	650.00	\$	1,800		
	LAND ACQUISTION/ PERMANENT DAMAGES	AC	1.49	\$	7,500.00	\$	11,180 6,627		
COUNTY ADMINISTRATION COSTS									
TOPOGRAPHIC SURVEY									
REPORTS, PLANS AND SPECIFICATIONS									
	CONSTRUCTION STAKING & ADMINISTRATION								
	TOTA	L TWO-S	TAGE DITCH	RE	PAIR COST	\$	206,906		



Proposed 31" x 51" Arch RCP Pipe Option

31-INCH X 51-INCH RCP ARCH PIPE AND WATERWAY

Item No.	Item	Unit	Quantity	Į	Jnit Price		Amount	
101	MOBILIZATION	LS	1	\$	14,820.00	\$	14,820	
102	31-INCH X 51-INCH ARCH RCP PIPE	LF	1040	\$	255.00	\$	265,200	
103	31-INCH X 51-INCH ARCH RCP PIPE APRON	EA	2	\$	2,650.00	\$	5,300	
104	FILL OPEN DITCH	CY	4160	\$	2.00	\$	8,320	
105	GRANULAR PIPE FOUNDATION	CY	241	\$	21.60	\$	5,200	
106	CONNECT EXISTING TILE (SIZE & MATERIAL MAY VARY)	EA	5	\$	500.00	\$	2,500	
107	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	150	\$	65.80	\$	9,870	
108	WATERWAY SEEDING	AC	3.1	\$	3,450.40	\$	10,806	
100	(SEED MIX: BUFFER BLEND WITH TYPE 8 MULCH)	A)	3.1	Ф	3,450.40	9	10,000	
109	SLOUGH #5 REPAIR	LS	1	\$	5,720.00	\$	5,720	
110	TREE REMOVALS	LS	1	\$	3,960.00	\$	3,960	
					TOTAL	\$	331,696	
			10%	∥ Ul	NFORSEEN	\$	33,170	
					SUBTOTAL	\$	364,866	
	TEMPORARY DAMAGES	AC	1.36	\$	650.00	\$	890	
	LAND ACQUISTION/ PERMANENT DAMAGES	AC	0.00	\$	7,500.00	\$	- 4,244	
COUNTY ADMINISTRATION COSTS								
TOPOGRAPHIC SURVEY								
REPORTS, PLANS AND SPECIFICATIONS								
CONSTRUCTION STAKING & ADMINISTRATION								
	TOTAL 31-INCH X 51-INCH RCP ARCH F	PIPE AND	WATERWAY	RE	PAIR COST	\$	431,871	



Proposed 42" RCP Pipe Option

42-INCH RCP PIPE AND WATERWAY

Item No.	Item	Unit	Quantity	Ī	Unit Price		Amount	
101	MOBILIZATION	LS	1	\$	11,130.00	\$	11,130	
102	42-INCH CLASS III RCP PIPE	LF	1040	\$	185.00	\$	192,400	
103	42-INCH RCP APRON	EA	2	\$	2,185.00	\$	4,370	
104	FILL OPEN DITCH	CY	4160	\$	2.00	\$	8,320	
105	GRANULAR PIPE FOUNDATION	CY	231	\$	21.60	\$	4,992	
106	CONNECT EXISTING TILE (SIZE & MATERIAL MAY VARY)	EA	5	\$	500.00	\$	2,500	
107	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	150	\$	65.80	\$	9,870	
108	WATERWAY SEEDING	AC	3.1	\$	3,450.40	\$	10,806	
100	(SEED MIX: BUFFER BLEND WITH TYPE 8 MULCH)	AC	3.1	Φ	3,450.40	Φ	10,600	
109	SLOUGH #5 REPAIR	LS	1	\$	5,720.00	\$	5,720	
110	TREE REMOVALS	LS	1	\$	3,960.00	\$	3,960	
					TOTAL	\$	254,068	
			109	% U	NFORSEEN	\$	25,407	
					SUBTOTAL	\$	279,475	
	TEMPORARY DAMAGES	AC	1.36	\$	650.00	\$	890	
	LAND ACQUISTION/ PERMANENT DAMAGES	AC	0.00	\$	7,500.00	\$	3,974	
COUNTY ADMINISTRATION COSTS								
TOPOGRAPHIC SURVEY								
REPORTS, PLANS AND SPECIFICATIONS								
CONSTRUCTION STAKING & ADMINISTRATION								
	TOTAL 42-INCH RCP F	PIPE AND	WATERWAY	RE	PAIR COST	\$	31,332 345,824	



Proposed 42" Dual Wall HDPE Pipe Option

42-INCH HDPE AND WATERWAY

Item No.	Item	Unit	Quantity	Į	Jnit Price		Amount	
101	MOBILIZATION	LS	1	\$	6,750.00	\$	6,750	
102	42-INCH AGRICULTURAL TILE	LF	1100	\$	100.00	\$	110,000	
103	42-INCH RCP APRON	EA	2	\$	2,185.00	\$	4,370	
104	FILL OPEN DITCH	CY	4400	\$	2.00	\$	8,800	
105	GRANULAR PIPE FOUNDATION	CY	278	\$	21.60	\$	6,005	
106	CONNECT EXISTING TILE (SIZE & MATERIAL MAY VARY)	EA	5	\$	500.00	\$	2,500	
107	CLASS III RIPRAP WITH GEOTEXTILE FABRIC	CY	50	\$	65.80	\$	3,290	
108	WATERWAY SEEDING	AC	3.1	\$	3,450.40	\$	10 906	
100	(SEED MIX: BUFFER BLEND WITH TYPE 8 MULCH)	AC	3.1	Ф	3,450.40	Ф	10,806	
109	SLOUGH #5 REPAIR	LS	1	\$	5,720.00	\$	5,720	
110	TREE REMOVALS	LS	1	\$	3,960.00	\$	3,960	
					TOTAL	\$	162,201	
			10%	/U %	NFORSEEN	\$	16,220	
					SUBTOTAL	\$	178,421	
	TEMPORARY DAMAGES	AC	1.36	\$	650.00	\$	890	
	LAND ACQUISTION/ PERMANENT DAMAGES	AC	0.00	\$	7,500.00	\$	-	
COUNTY ADMINISTRATION COSTS								
TOPOGRAPHIC SURVEY								
REPORTS, PLANS AND SPECIFICATIONS								
CONSTRUCTION STAKING & ADMINISTRATION								
	TOTAL 42-INCH HI	OPE AND	WATERWAY	RE	PAIR COST	\$	244,086	